



نگهداری و افزایش تولید میدان نفتی بینک سطح الارض

احداث ردیف تراکم گاز در ایستگاه جمع آوری پینک



شماره پیمان:

Calculation Note For CCTV Control Room- Binak GCS

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طرح نگهداری و افزایش تولید ۲۷ مخزن

Calculation Note For CCTV Control Room- Binak GCS

نگهداشت و افزایش تولید میدان نفتی بینک

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1.0 INTRODUCTION

Binak oilfield in Bushehr province is a part of the southern oilfields of Iran, is located 20 km northwest of Genaveh city.

With the aim of increasing production of oil from Binak oilfield, an EPC/EPD Project has been defined by NIOC/NISOC and awarded to Petro Iran Development Company (PEDCO). Also PEDCO (as General Contractor) has assigned the EPC-packages of the Project to "Hirgan Energy - Design and Inspection" JV.

As a part of the Project, construction of well location, access road, wellhead facilities (with electric power supply) for W007S shall be done. In addition, construction of new flowline from aforementioned well location to Binak B/C unit (with extension of relevant manifold) are in the Project scope of work

2.0 SCOPE

This report covers designing of structure & foundation calculations of the “CCTV Control Room-Binak GCS”. The structure calculation is performed by “ETABS” & calculation of foundation is performed by “SAFE” software's.

3.0 NORMATIVE REFERENCE

3.1 Local Codes and Standards

- INBC Part 6 “Iranian National Building Code
- INBC Part 7 “Iranian National Building Code
- INBC Part 9 “Iranian National Building Code
- INBC Part 10 “Iranian National Building Code
- Iranian Seismic Design Code for Petroleum Facilities(3rd edition)

3.2 International Codes and Standards

- ASCE 7-10 “Minimum Design Loads and Associated Criteria for Buildings and Other Structures-American Society of Civil Engineers”.
- ACI 318. “Building Code Requirements for Reinforced Concrete”, American Concrete Institute.
- AISC 358 “Prequalified Connections for Special and Intermediate Steel Moment Frames for Seismic Applications.” American Institute of Steel Construction, Inc.
- AISC 360 - “Specification for Structural Steel Buildings”. American Institute of Steel Construction, Inc.



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3.3 The Project Documents

- BK-GNRAL-PEDCO-000-ST-SP-0001 SPECIFICATION FOR CONCRETE WORK
- BK-GNRAL-PEDCO-000-ST-DC-0001 Structural Design Criteria
- BK-GNRAL-PEDCO-000-CV-SP-0004 Specification For Earth Work
- BK-GCS-PEDCO-120-GT-RT-0001 Geotechnical Investigation Report for Compressor Station
- BK-GCS-PEDCO-120-AR-DW-0008 Architectural Drawing For CCTV Control Room- Binak GCS

4.0 MATERIAL PROPERTIES

Material properties are delivered in the following table.

Material properties	
Structure and Foundation concrete	F'c=300kg/cm ² (28 days cylindrical sample)
Long. Reinforcement	Fy=4000 kg/cm ² (AIII)
Trans. Reinforcement	Fy=4000 kg/cm ² (AIII)

5.0 COMPUTER SOFTWARE

Computer's Software, which is used in structure and foundation analysis and design, are defined in the following table.

Computer software	
analysis and design of structure	ETABS 16.2.1
analysis and design of foundation	SAFE 16.0.2

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6.0 DESIGN INFORMATION

STRUCTURE LOCATION

The CCTV control room is located in Binak oilfield.

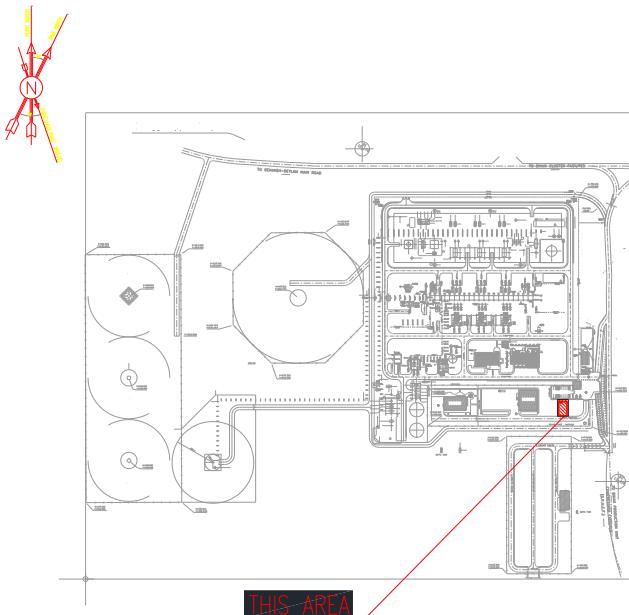


Figure 1: Project Location

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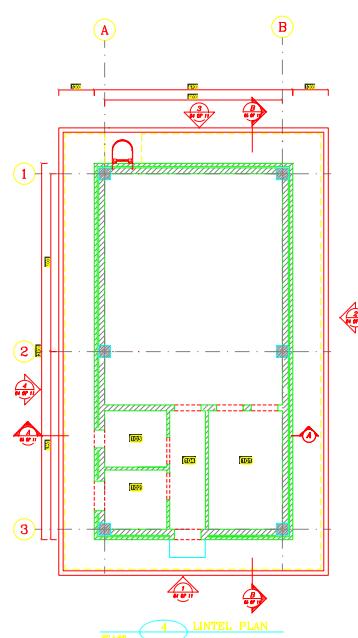


Figure 2: Plan of CCTV control room

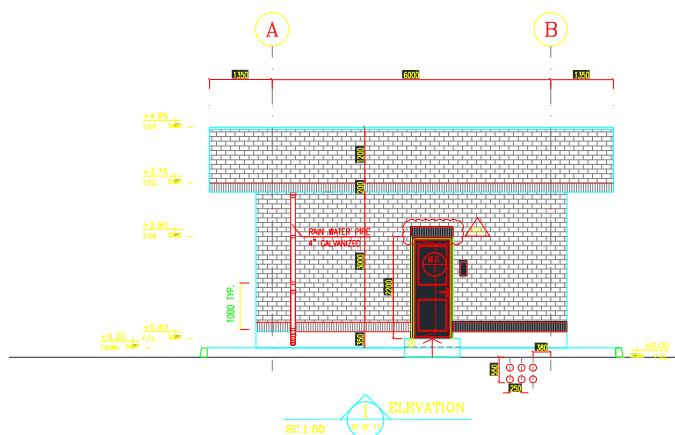


Figure 3: Elevation 1

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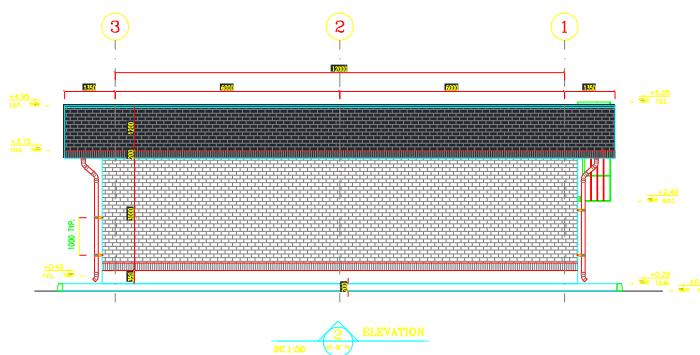


Figure 4: Elevation 2

7.0 MATERIAL PROPERTIES

○ REINFORCED CONCRETE

Concrete shall generally conform to the specification for Concrete Work, Document No:

BK-GNRAL-PEDCO-000-ST-SP-0001. The following properties of concrete are used.

Lean concrete: $f'_c = 150 \text{ kg/cm}^2$

Cast in place concrete: $f'_c = 300 \text{ kg/cm}^2$

Where f'_c is the minimum compressive characteristic strength of a cylinder specimen at 28 days.

Young Modulus of concrete: $E_c = 15100\sqrt{f'_c} \text{ kg/cm}^2 = 261540 \text{ kg/cm}^2$

Poisson's Ratio: $\nu = 0.2$

Unit weight of reinforced concrete: 2500 kg/m^3

Reinforcing Steel: $f_y = 4000 \text{ kg/cm}^2$ (Minimum yield stress)



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○ STIFFNESS MODIFICATION

For analysis of concrete structure, the following modifications for flexural stiffness of elements are considered.

Columns.....0.7lg

Beams.....0.35lg

8.0 DESIGN LOADS

○ GENERAL

Structural elements stability and stress checking have been performed considering the following load conditions.

- Dead Load
- Live Load
- Seismic Load
- Snow Load

9.0 DEAD LOAD

○ FOR CCTV CONTROL ROOM

Dead Load is considered as the weight of materials forming a permanent part of the structure plant. The weight of materials of construction incorporated into the building, including but not limited to walls, floors, roofs, ceilings, stairways, built-in partitions, finishes, cladding and other similarly incorporated architectural and structural items, and the weight of fixed service equipment, such as plumbing stacks and risers, electrical feeders, heating, ventilating and air-conditioning systems.

Specific weight of materials which will be used is based on Iranian National Building Code, Part 6, where applicable. Other weights are in accordance with the specifications and/or drawings of vendors and manufacturers.

As it mentioned above the self-weight of structural elements (introduced Dead Load in software) is automatically considered by ETABS program with the specific weights below:

Reinforced Concrete: 2500 kg/m³

Structural Steel and Bars: 7850 kg/m³



Here is the calculation of design dead load for floors and walls.

- dead load for parapet wall(for 120cm height) :

(Pressed brick = $20\text{cm} \times 1700 = 340 \rightarrow 340 \times 1.2 = 408 \text{ kg/m}$) + (Travertine stone= $2500 \times 2\text{cm} = 50 \rightarrow 50 \times 1.2 = 60 \text{ kg/m}$) + (mortar = $1\text{cm} \times 2100 = 21 \rightarrow 21 \times 1.2 = 25.2 \text{ kg/m}$) + (Terrazzo tile ($30^*30^*2.5$)= $2500 \times 2\text{cm} = 50 \rightarrow 50 \times 0.3 = 15 \text{ kg/m}$) + (mortar = $1\text{cm} \times 2100 = 21 \rightarrow 21 \times 0.3 = 6.3 \text{ kg/m}$) + (cement = $3\text{cm} \times 2100 = 63 \rightarrow 63 \times 1 = 63 \text{ kg/m}$) + (Waterproofing (Isolation or Similar = $15 \text{ kg/m}^2 \times 30\text{cm} = 4.5 \text{ kg/m}$) → totally ≈ 585 kg/m

- dead load for wall (for supper dead load(wall)) :

(Face Brick = 5cm × 1700 = 85) + (Pressed brick = 20cm × 1700 = 340) + (plaster and soil mortar = 3cm × 1600 = 48) → totally ≈ 475 kg/m²

- dead load for roof :

(mosaic = $2.5\text{cm} \times 2250 = 56.5$) + (mortar = $2.5\text{cm} \times 2100 = 52.5$) + +(pumice = $9\text{cm} \times 600 = 54$) + (clay block = $8 \times 10 = 80$) + (facilities = 20) + (waterproofing = 15) → totally ≈ 300 kg/m²

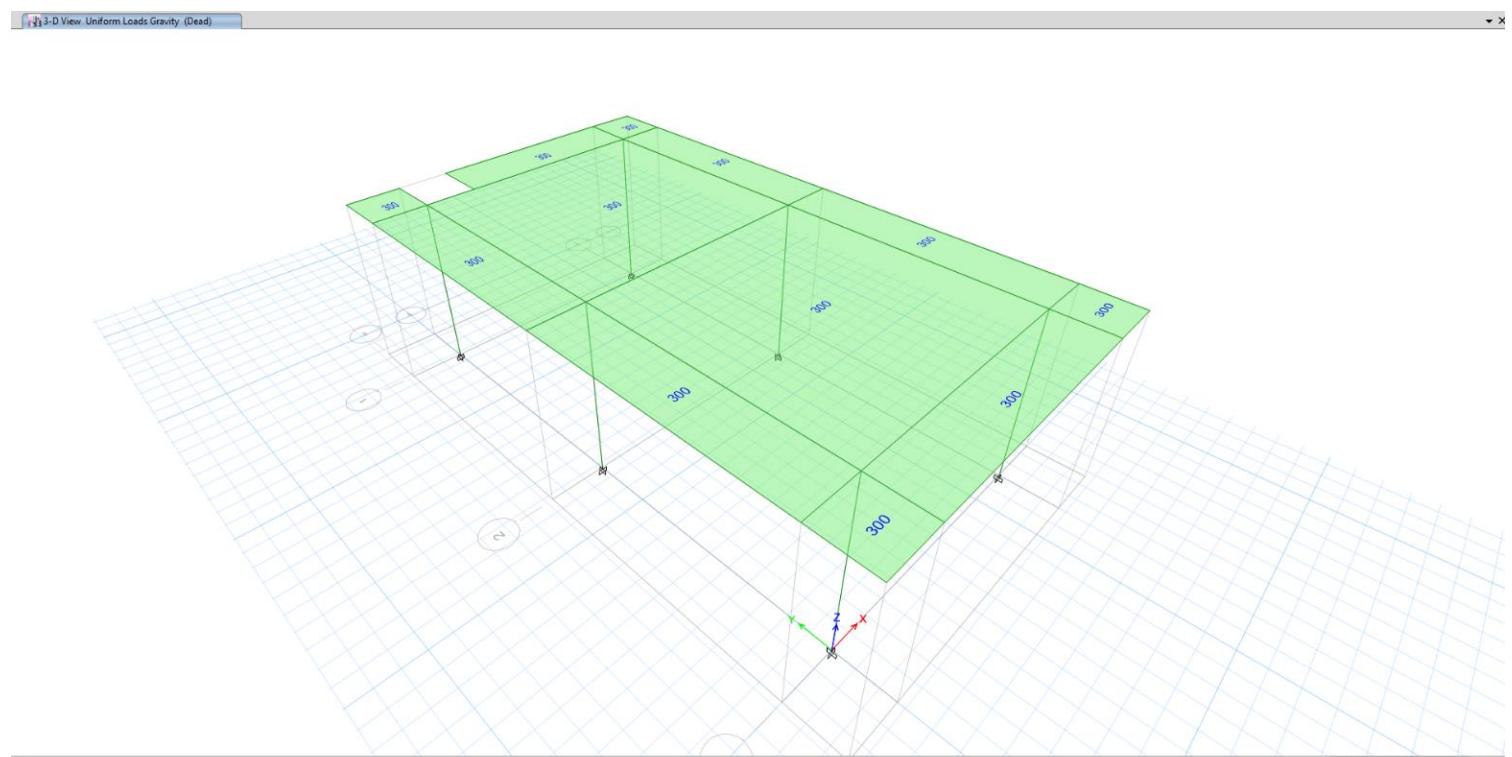


Figure 5: Dead load on roof

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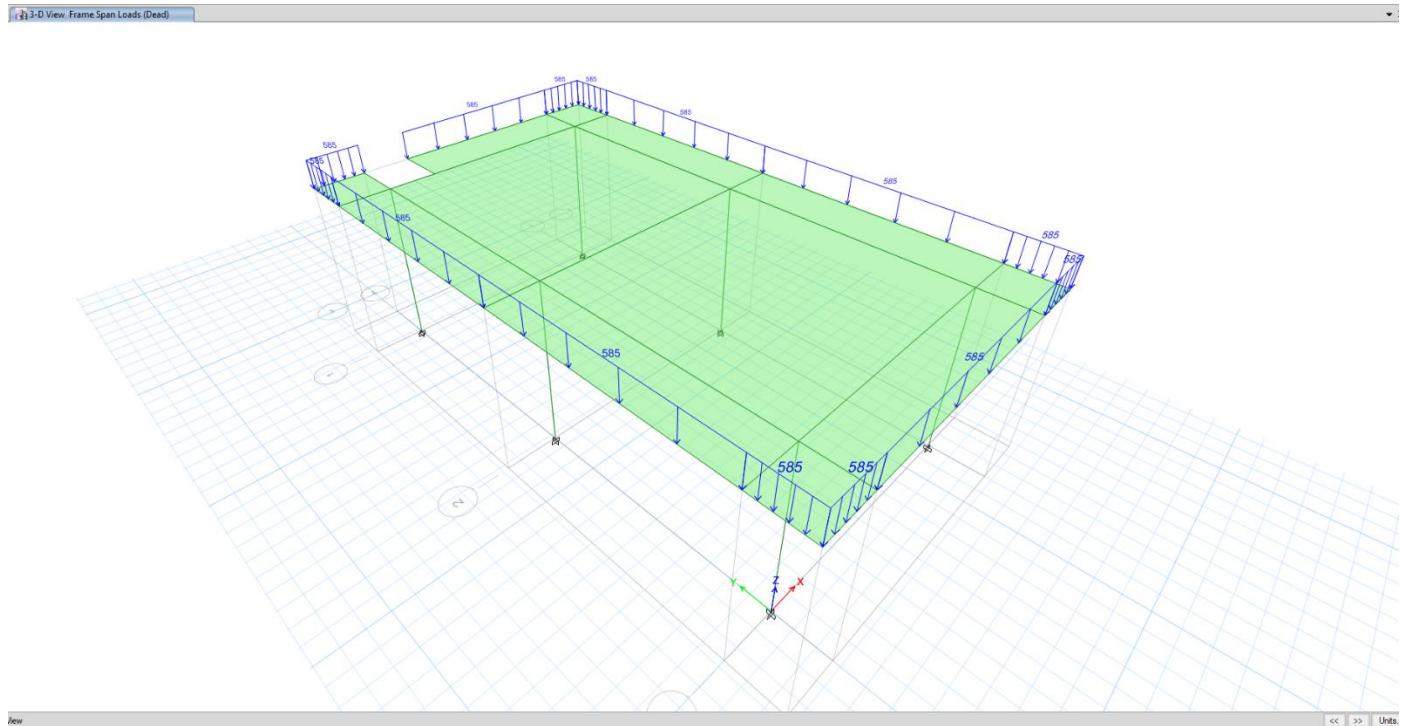


Figure 6: Parapet dead load

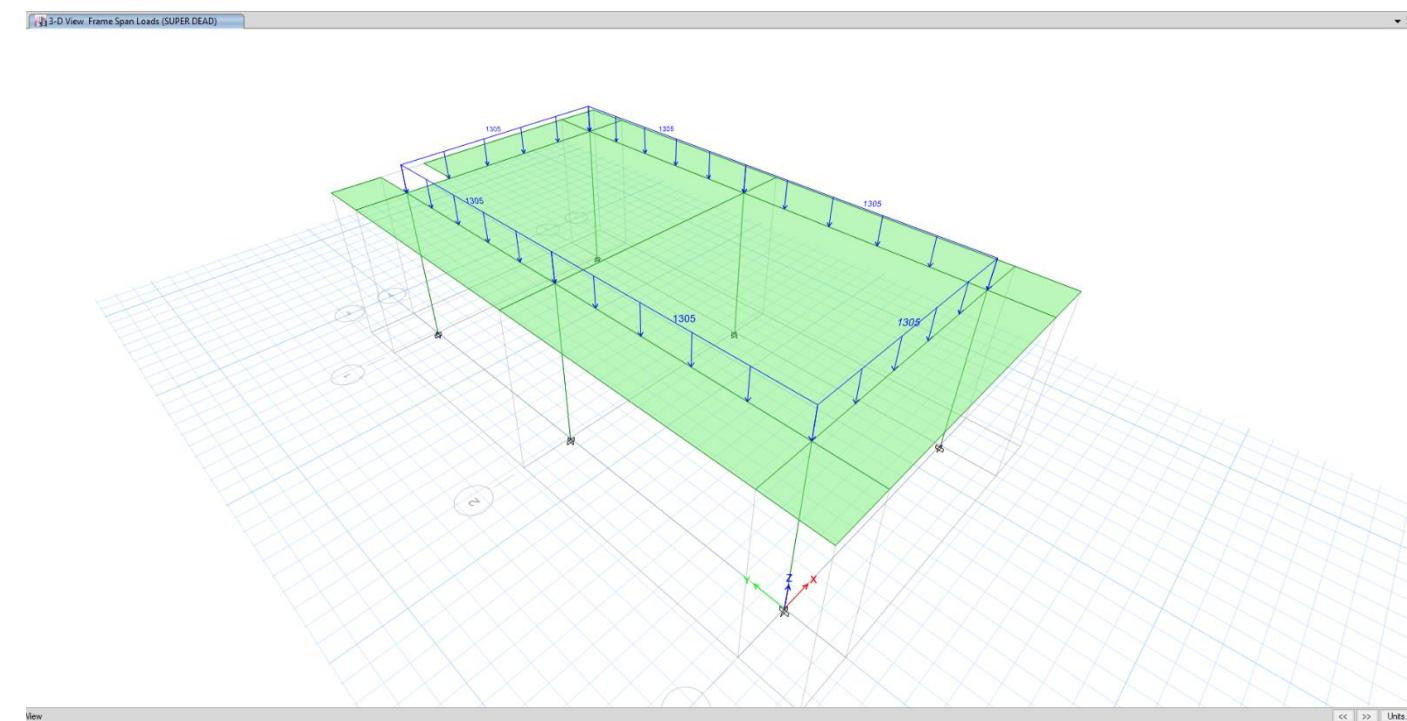


Figure 7: Dead load for wall

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10.0 LIVE LOAD

Live Load is defined as the weight of all movable loads, including partition walls, personnel, tools, miscellaneous equipment and temporarily stored materials.

Generally where applicable, the live loads shall be in accordance with Iranian National Building Code, Part 6. The Live Loads is generally considered as uniformly distributed over the horizontal projection of the loaded areas, except for the loads with a concentrated nature.

The live load has been considered according to the following table.

• **Table 1- Live Load**

NO.	Level	kg/m ²
1	Roof	150

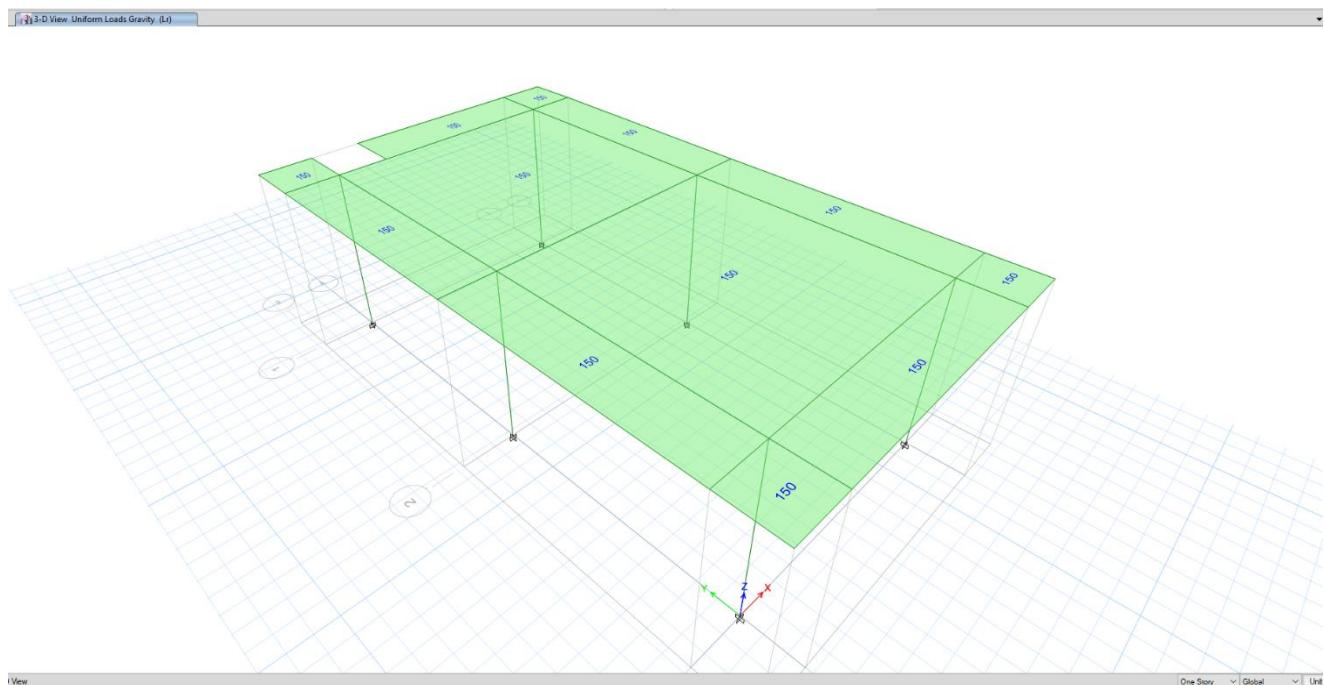


Figure 7: Live load on roof

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11.0 SNOW LOAD

FOR CCTV CONTROL ROOM

Snow load of this structure is calculated in accordance with Iranian National Building Code No.6 Latest edition. Parameters which are used in calculation of snow force is presented in below:

$$P_r = I_s \cdot C_n \cdot C_h \cdot C_s \cdot P_s \quad P_s = 25 \text{ kg/m}^2$$

$$I_s = 1, C_n = 1, C_h = 1, C_s = 1 \rightarrow P_r = P_s = 25 \text{ kg/m}^2$$

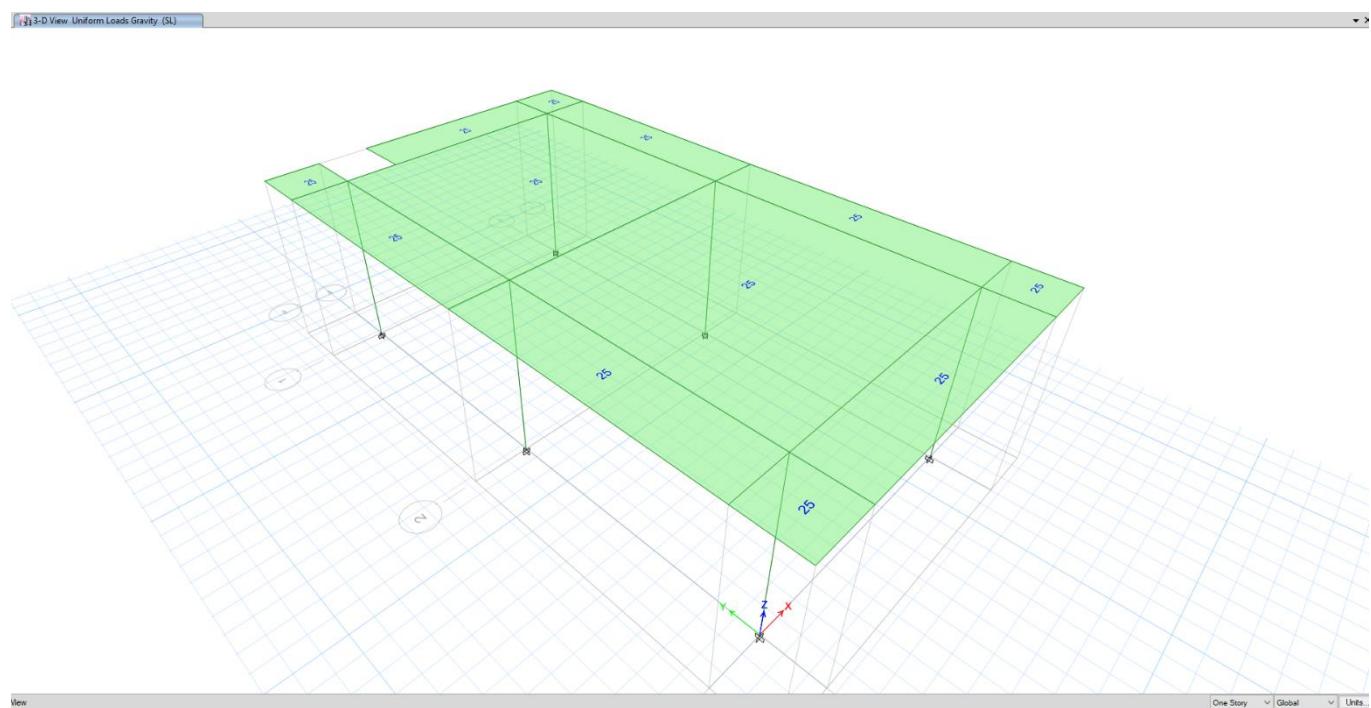


Figure 8: Snow load on roof

12.0 SEISMIC LOAD

HORIZONTAL SEISMIC LOAD

According to Iranian Seismic Design Code for Petroleum Facilities (Pub.038-3rd edition) the structure shall be designed for earthquake load in two orthogonal directions.

Base level is defined as the level below which the structure does not move relative to the ground during an earthquake.

Basic parameters which are used in calculation of earthquake forces are presented below.



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Following formula is used for calculations according to Iranian Seismic Design Code for Petroleum Facilities (Pub.038-3rd edition)

$$V_u = \frac{S_a}{R_u/I} W$$

In which:

V_u : Basic Shear

S_a : Mapped Spectral Response Acceleration Parameter (g)

I : Importance Factor of Structure

R_u : Structural System Factor

W = Effective Seismic Weight of the structure, including dead loads and other loads, calculated from base level.

• **Table 6- B Table 6- Basic Parameters Used for Earthquake Loads**

Height of the structure from the base level (m)	4.35
Importance factor, I	1.25
Structural System	Special Moment Frame at Both Direction
Soil type	II
R_u	8
C_t (Based on table (4-6) Pub.038)	0.047
X (Based on table (4-6) Pub.038)	0.9
$T_{x\&Y} (calculation) = C_t \cdot H^x$	0.176
Based on part (4-8-3) Pub.038 : $T_{x\&Y} (calculation)$	= 0.176 * 1.4 = 0.247
T_y (analysis) – mode 1	0.59
T_x (analysis) – mode 2	0.58
T_x (select for design)	0.247
T_y (select for design)	0.247
Sa_x	0.75



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Sa_y	0.75
$C_{min} = 0.044S_{DS}I$	0.04125
$C_x = \frac{S_{ax}}{R_u/I}$	0.136
$C_y = \frac{S_{ay}}{R_u/I}$	0.136

SD1	Sa	T0	Ts
0.375	0.750	0.1	0.5

○ VERTICAL SEISMIC LOAD

Ev : Vertical seismic load applied at model according to section 2-2-3-2 (code 038)

$$E_v = \alpha S_{DS} D = 0.2 * 0.75D = 0.15D$$

Load Case Data

General		
Load Case Name	Ev	
Load Case Type	Linear Static	
Exclude Objects in this Group	Not Applicable	
Mass Source	MsSrc	
P-Delta/Nonlinear Stiffness		
<input checked="" type="radio"/> Use Preset P-Delta Settings	Iterative based on loads	
<input type="radio"/> Use Nonlinear Case (Loads at End of Case NOT Included)	Nonlinear Case	
Loads Applied		
Load Type	Load Name	Scale Factor
Load Pattern	Dead	0.15
		Add
		Delete

OK Cancel



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13.0 REDUNDANCY FACTOR ρ

According to Iranian Seismic Design Code for Petroleum Facilities (Pub.038-3rd edition Paragraph 4-6) - $\rho = 1.0$

14.0 LOADING TABLE

Loading for structure and foundation for CCTV control room is defined as below table.

TABLE

Load Pat	Design Type	Self Wt Mult	Auto Load	Not Base Pat	Not Ratio	Not Dir.
DL	Dead	1				
Lr	Roof Live	0				
EX	Quake	0	USER COEFF			
EY	Quake	0	USER COEFF			
SL	Snow	0				
SUPERDEAD	Dead	0				
T	Temperature	0				
Ev	Quake	0	None			
NotionalX(DL)	Notional	0		DL	0.002	Global X
NotionalY(DL)	Notional	0		DL	0.002	Global Y
NotionalX(Lr)	Notional	0		Lr	0.002	Global X
NotionalY(Lr)	Notional	0		Lr	0.002	Global Y
NotionalX (SUPERDEAD)	Notional	0		SUPERDEAD	0.002	Global X
NotionalY (SUPERDEAD)	Notional	0		SUPERDEAD	0.002	Global Y
Soil	Other	0				



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Notional loads must be added with the same coefficient in all combinations of loads that have gravity loads. Although this type is not necessary for concrete building, it has been considered.

15.0 LOAD COMBINATIONS

For foundations, structures and members of structures, according to structural design criteria & Iranian National Building Code Part 6, the following load combinations have been considered:

- 1) 1.4D
- 2) 1.2D + 1.6L + 0.5 (Lr or S)
- 3) 1.2D + 1.6 (Lr or S) + (L or 0.5(1.6W))
- 4) 1.2D + 1.6 W + L + 0.5 (Lr or S)
- 5) 1.2D+(Eh + Ev)+ L + 0.2S
- 6) 0.9D + 1.6W
- 7) 0.9D + (Eh – Ev)

Load listed herein shall be considered to act in the following combinations; whichever produces the most unfavourable effect considering soil reactions.



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Calculation Note For CCTV Control Room- Binak GCS

پروژه	بسته کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	سربال	نسخه
BK	GCS	PEDCO	120	ST	CN	0027	D00

Allowable Stress Design:

Category		Load Combination
Category A	Operation Without Wind	1.0(DL+ DLempty + LLop)
		1.0(DL+ DLempty + LLop + LL + TL ± TLst ± FR)
		1.0(DL+ DLempty + LLop + TL ± TLst ± FR) + 0.75LL + 0.75S
	Operation With Wind	1.0(DL+ DLempty + LLop + TL ± FR) ± WL
		1.0(DL+ DLempty + LLop + TL ± FR) + 0.75LL ± 0.75WL
		1.0(DL+ DLempty + LLop + TL ± FR) + 0.75LL ± 0.75WL + 0.75S
Category B	Test	0.6(DL+ DLempty + LLop+TL + FR) ± WL
		1.0(DL+ DLempty + Test) ± 0.25WL
		1.0(DL+ DLempty + Test) + 0.75LL ± 0.25WL + 0.75S
Category C	Erection	0.6(DL+ DLempty + Test) ± 0.25WL
		1.0(DL+ DLempty + ER ± WL)
		1.0(DL+ DLempty + ER) + 0.75LL ± 0.75WL
Category D	Earthquake	0.6(DL+ DLempty + ER) ± WL
		1.0(DL+ DLempty + LLop + TL) ± 0.7EQ
		1.0(DL+ DLempty + LLop + TL) ± 0.525EQ + 0.75LL + 0.75S
Category E	Maintenance	0.6(DL+ DLempty + LLop+ TL) ± 0.7EQ
		1.0(DL+ DLempty + ML)
		1.0(DL+ DLempty+ ML) + 0.75LL ± 0.25WL
		0.6(DL+ DLempty+ ML) ± 0.25WL



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Calculation Note For CCTV Control Room- Binak GCS

پروژه	بسته کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	سربال	نسخه
BK	GCS	PEDCO	120	ST	CN	0027	D00

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Strength Design:

Category		Load Combination
Category A	Operation Without Wind	1.4(DL+ DLempty + LLop)
		1.2(DL+ DLempty + LLop ± FR + TL ± TLst) +1.6LL
		1.2(DL+ DLempty + LLop ± FR + TL ± TLst) +1.6LL + 0.5S
	Operation With Wind	1.2(DL+ DLempty + LLop ± FR + TL) + 1.0LL ± 1.6WL + 0.5S
		1.2(DL+ DLempty + LLop ± FR + TL) ± 0.8WL + 1.6S
		0.9(DL+ DLempty + LLop ± FR + TL) ± 1.6WL
Category B	Test	1.2(DL+ DLempty + Test) + 1.0LL ± 1.6(0.25WL) + 0.5S
		1.2(DL+ DLempty + Test) + 1.6LL + 0.5S
		0.9(DL+ DLempty + Test) ± 1.6(0.25WL)
Category C	Erection	1.2(DL+ DLempty + ER) + 1.6LL
		1.2(DL+ DLempty + ER) + 1.0LL ± 0.8WL
		0.9(DL+ DLempty + ER) ± 1.6WL
Category D	Earthquake	1.2(DL+ DLempty + LLop + TL) + 1.0LL ± 1.0EQ + 0.2S
		0.9(DL+ DLempty + LLop+ TL) ± 1.0EQ
Category E	Maintenance	1.2(DL+ DLempty + ML) + 1.0LL ± 1.6(0.25WL)
		1.2(DL+ DLempty + ML) + 1.6(LL)
		0.9(DL+ DLempty+ ML) ± 1.6(0.25WL)

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Calculation Note For CCTV Control Room- Binak GCS

پروژه	بسته کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	نسخه
BK	GCS	PEDCO	120	ST	CN	0027
					D00	

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16.0 STRUCTURE ANALYSIS AND DESIGN

The steel structure is checked in accordance with LRFD method. Frame analysis and structural checks are based on the 3D model that covers all the Load Combinations.

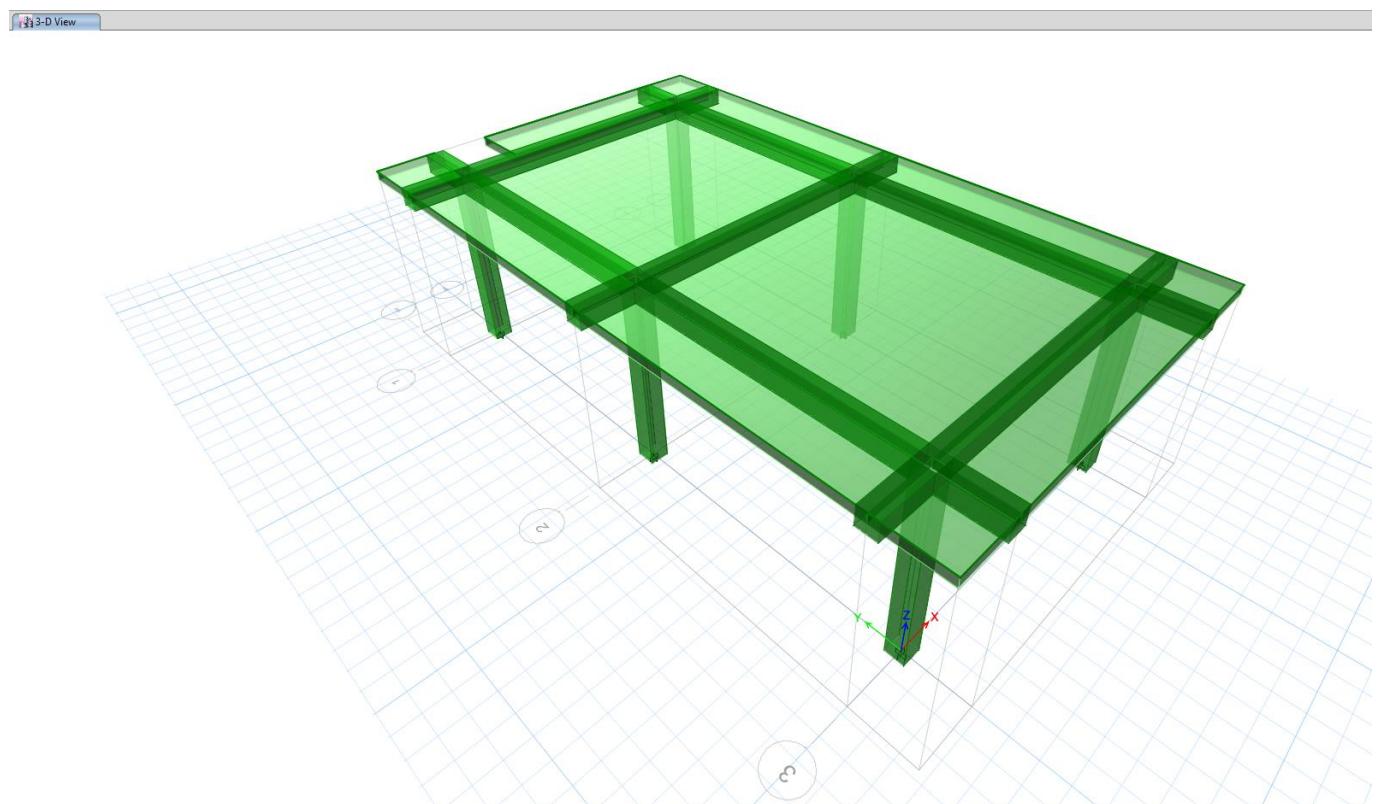


Figure 9: 3D model of ETABS

شماره پیمان:

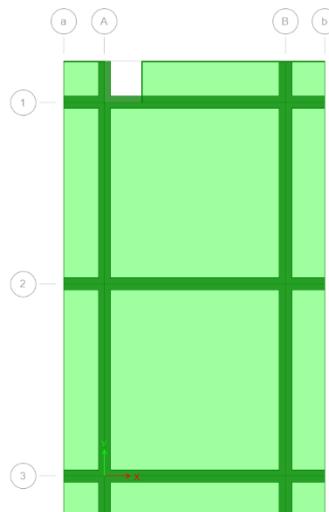
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Calculation Note For CCTV Control Room- Binak GCS

شماره صفحه: ۲۱ از ۳۰

پروژه	بسته کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	سریال	نسخه
BK	GCS	PEDCO	120	ST	CN	0027	D00

Plan View - 1 - Z = 435 (cm)



X 1325 Y 1695 Z 435 (cm) One Story Global Unit

Figure 10: Plan View

17.0 STRUCTURAL DESIGN RESULTS

The design of these elements is done by software facilities according to Code ACI-318-14. Summary of the results for columns and beams design is shown in the following figures.

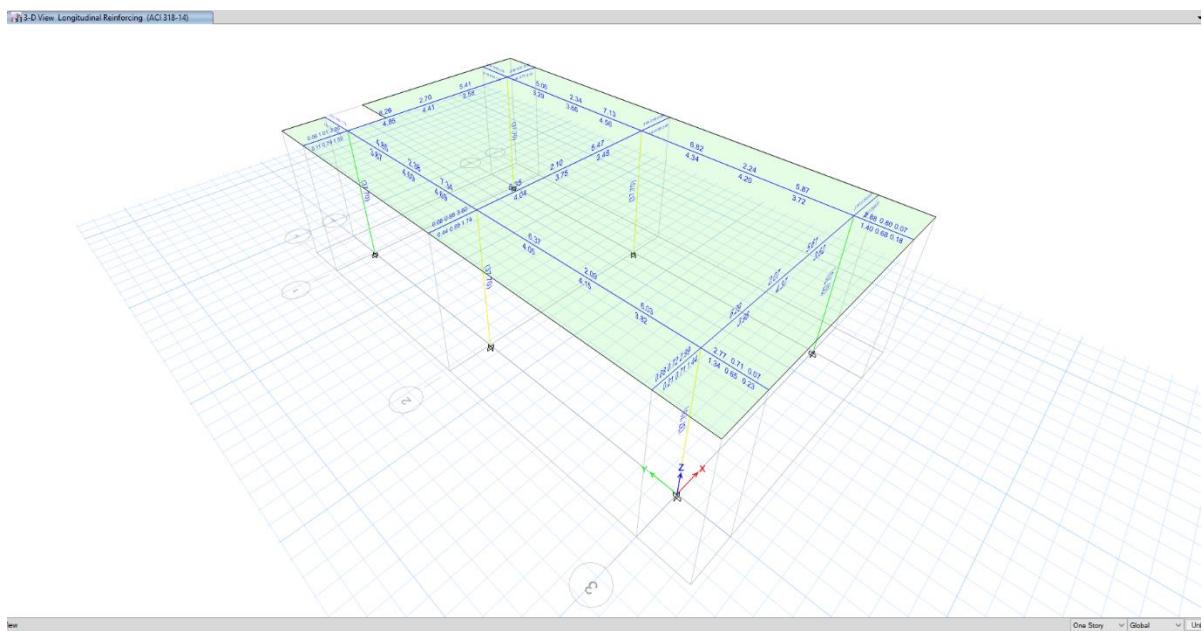


Figure 11: Ratio Check



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Calculation Note For CCTV Control Room- Binak GCS

پروژه	بسته کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	نسخه
BK	GCS	PEDCO	120	ST	CN	0027
			D00			

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For all elements D/C ratio shall be ≤ 1 , according to above output all elements are ok.

18.0 DRIFT CONTROL

According to "Iranian Seismic Design Code for Petroleum Facilities (Pub.038-3rd edition)" Table 4-8, drift shall not exceed 0.02.

گروه کاربری و خطرزاوی			انواع سازه‌ها
IV	III	II و I	
۰/۰۱۵	۰/۰۲۰	۰/۰۲۵	سازه‌های چهار طبقه و کمتر با تیغه‌بندی‌ها، سقف‌ها، دیوارهای داخلی و سیستم دیوارهای جانبی پیرامونی بدون دیوار برپی شنای که در برابر جابجایی نسبی طبقه طراحی شده‌اند.
۰/۰۱۰	۰/۰۱۰	۰/۰۱۰	سازه‌های با دیوار برپی بنایی طرهای
۰/۰۰۷	۰/۰۰۷	۰/۰۰۷	دیگر سازه‌های با دیوار برپی بنایی
۰/۰۱۰	۰/۰۱۵	۰/۰۲۰	دیگر سازه‌ها

The deflection at level X (δ_x) (in. or mm) used to compute the design story drift, Δ , shall be determined in accordance with the following equation:

Equation 4-22 (Pub.038-3rd edition, part 4-14-1):

δ_{xe} = Maximum displacement in x direction due to earthquake: 11.9 mm

$$\delta_x = \frac{C_d \delta_{xe}}{I} = \frac{5.5 \times 1.19}{1.25} = 5.23 \text{ cm} \rightarrow \Delta_x = \frac{5.23}{435} = 0.012 < 0.02 \text{ ok}$$

Also

δ_{ye} = Maximum displacement in y direction due to earthquake: 11.6mm

$$\delta_y = \frac{C_d \delta_{ye}}{I} = \frac{5.5 \times 1.16}{1.25} = 5.10 \text{ cm} , \quad \Delta_y = \frac{5.10}{435} = 0.011 < 0.02 \text{ ok}$$

19.0 JOIST SHEAR CAPACITY RATIO CONTROL

According to ACI 318-14 part 18-8-4, shear strength of connection has been controlled and for two columns (Axis 4C & D1) as a sample have been shown below.

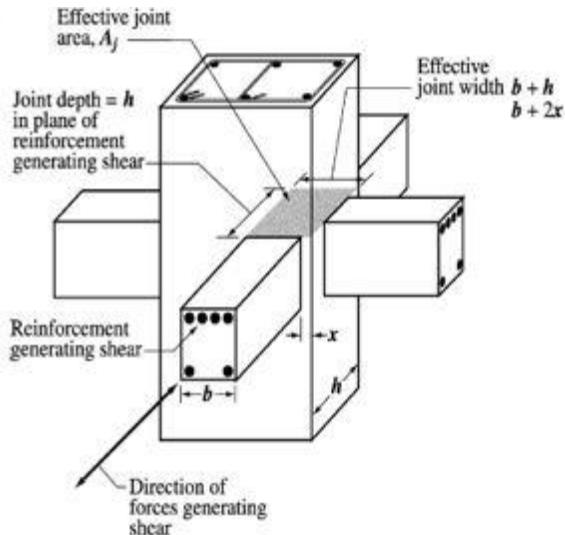
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Calculation Note For CCTV Control Room- Binak GCS

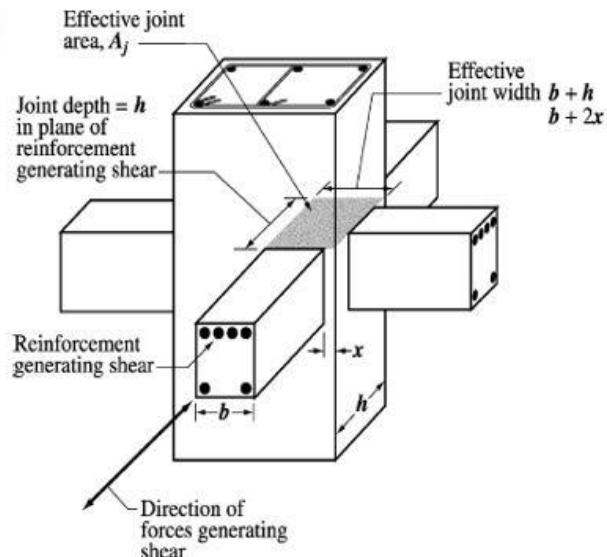
شماره صفحه: ۲۳ از ۳۰

پروژه	بسه کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	سریال	نسخه
BK	GCS	PEDCO	120	ST	CN	0027	D00

Location:	At the Intersection of Axes 4&C
$f_y =$	4000.00 kg/cm ²
$f'_c =$	300.00 kg/cm ²
$h =$	45.00 cm
$B =$	45.00 cm
$A_{s1} =$	10.160 cm ²
$A_{s2} =$	10.160 cm ²
$A_s =$	$A_{s1} + A_{s2}$ cm ²
$A_s =$	20.32 cm ²
$b =$	45.00 cm
$x =$	0.00 cm
$\bar{\theta} =$	0.75
$A_j =$	$h(\min(b+h), (b+2x))$ cm ²
$A_j =$	2025 cm ²
$V_n =$	$5.3\bar{\theta}A_j(f'_c)^{0.5}$
$V_n =$	139.4 ton
$V_u =$	$1.25f_y A_s$
$V_u =$	101.6 ton
Ratio=	V_u / V_n
Ratio=	0.73 OK



Location:	At the Intersection of Axes D1
$f_y =$	4000.00 kg/cm ²
$f'_c =$	300.00 kg/cm ²
$h =$	45.00 cm
$B =$	45.00 cm
$A_{s1} =$	7.620 cm ²
$A_{s2} =$	7.620 cm ²
$A_s =$	$A_{s1} + A_{s2}$ cm ²
$A_s =$	15.24 cm ²
$b =$	45.00 cm
$x =$	0.00 cm
$\bar{\theta} =$	0.75
$A_j =$	$h(\min(b+h), (b+2x))$ cm ²
$A_j =$	2025 cm ²
$V_n =$	$3.2\bar{\theta}A_j(f'_c)^{0.5}$
$V_n =$	139.4 ton
$V_u =$	$1.25f_y A_s$
$V_u =$	76.2 ton
Ratio=	V_u / V_n
Ratio=	0.55 OK



20.0 STRONG COLUMN-WEAK BEAM REQUIREMENTS IN SPECIAL CONCRETE MOMENT FRAME

According to INBC Part 9 (section: 9-20-60-4), the flexural strength of columns shall satisfy:

$$\Sigma M_{nc} \geq 1.2 \Sigma M_{nb}$$

Based on Etabs report, maximum force due to seismic load combination is equal to 48.6 ton which

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Calculation Note For CCTV Control Room- Binak GCS

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BK	GCS	PEDCO	120	ST	CN	0027	D00

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is smaller than $0.1A_g f_c$, so according to section 9-20-6-4-5 could be ignored above equation for one story building.

21.0 FOUNDATION DESIGN AND RESULTS

○ FOUNDATION MODEL

Foundation model, analyse and design has been done by SAFE2016 software.

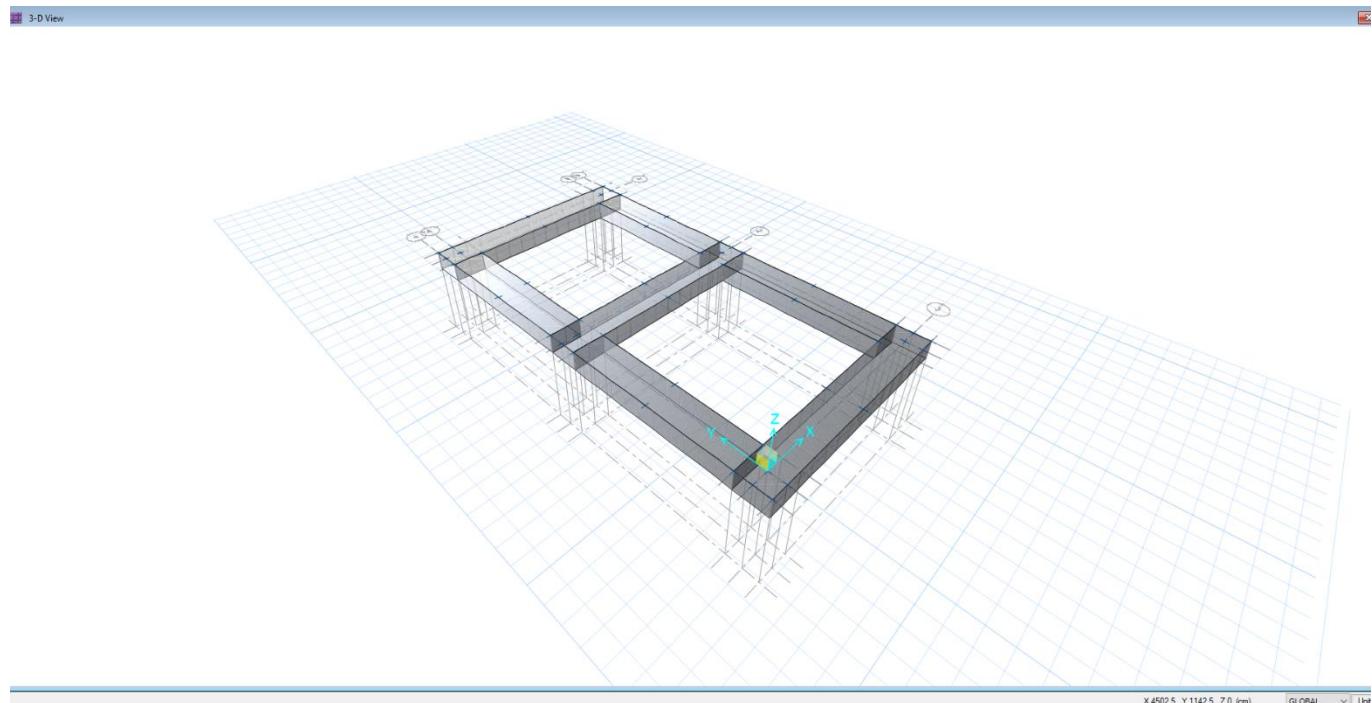


Figure 11: Foundation 3D plan in SAFE

○ SOIL CHARACTERISTIC

Based on Geotechnical Report (Attachment 4, 5):

$q_{\text{allowable}} = 1.75 \text{ kg/cm}^2$ (Allowable Soil Bearing Capacity)

$\delta_{\text{allowable}} = 1.6 \text{ cm}$ (Allowable Settlement)

$K_s = 1.6 \text{ kg/cm}^3$ (Subgrade Modulus)

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Calculation Note For CCTV Control Room- Binak GCS

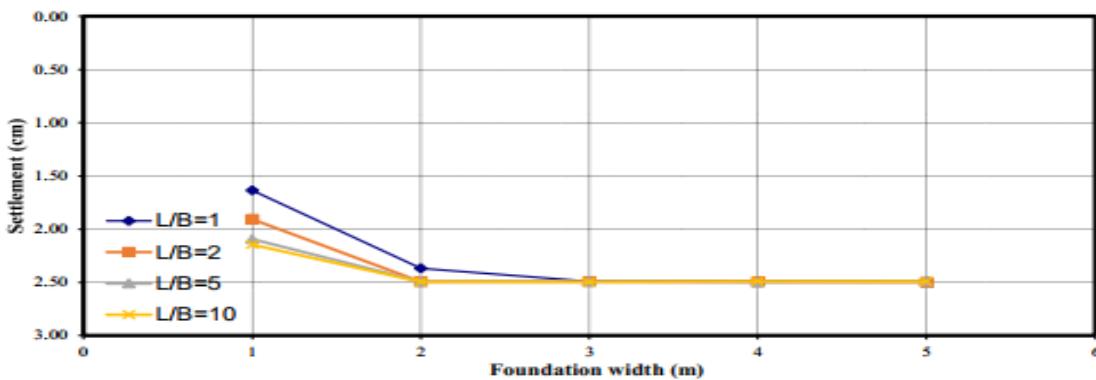
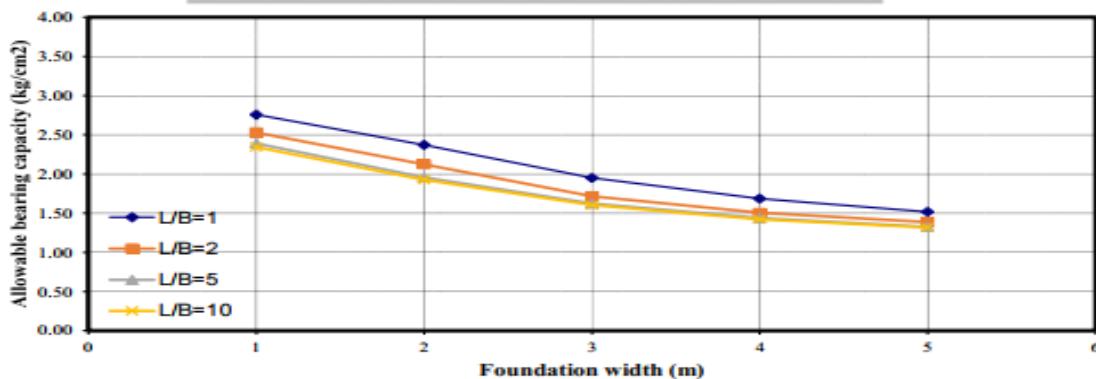
پروژه	بسه کاری	صادر کننده	تسهیلات	رشته	نوع مدرک	سربال	نسخه
BK	GCS	PEDCO	120	ST	CN	0027	D00

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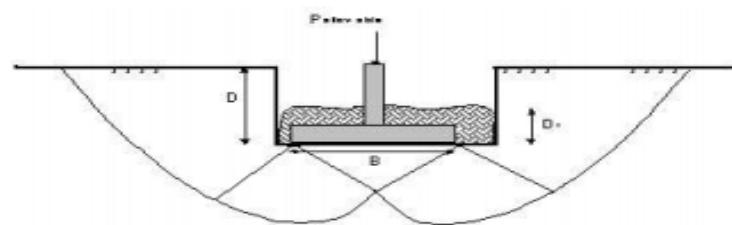
جدول 6-1. مدول عکس العمل بستر پی مربعی، مستطیلی و نواری برای عمق یک متر

B(m)	مدول عکس العمل بستر پی سطحی (kg/cm ³)			
	L/B=1	L/B=2	L/B=5	L/B=10
1.0	1.69	1.33	1.14	1.09
2.0	1.00	0.85	0.78	0.77
3.0	0.78	0.69	0.65	0.64
4.0	0.68	0.60	0.57	0.57
5.0	0.61	0.55	0.53	0.53

Shallow Foundation - GCS



$D = 1\text{m}$
 $D_f = 1\text{m}$



Notes:

D : Depth of footing with respect to ground surface

D_f : Depth of footing embedment

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Calculation Note For CCTV Control Room- Binak GCS

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LOADS

The loads have been imported from ETABS analysis.

Reinforced concrete unit weight equals 2500 kg/m^3 and defined for SAFE, so the program calculated the foundation weight automatically.

Soil unit weight equals to 1900 kg/m^3 . The soil height above foundation is 0.95 m, so distributed soil weight:

$$W_s = 0.95 \times 1900 = 1805 \text{ kg/m}^2$$

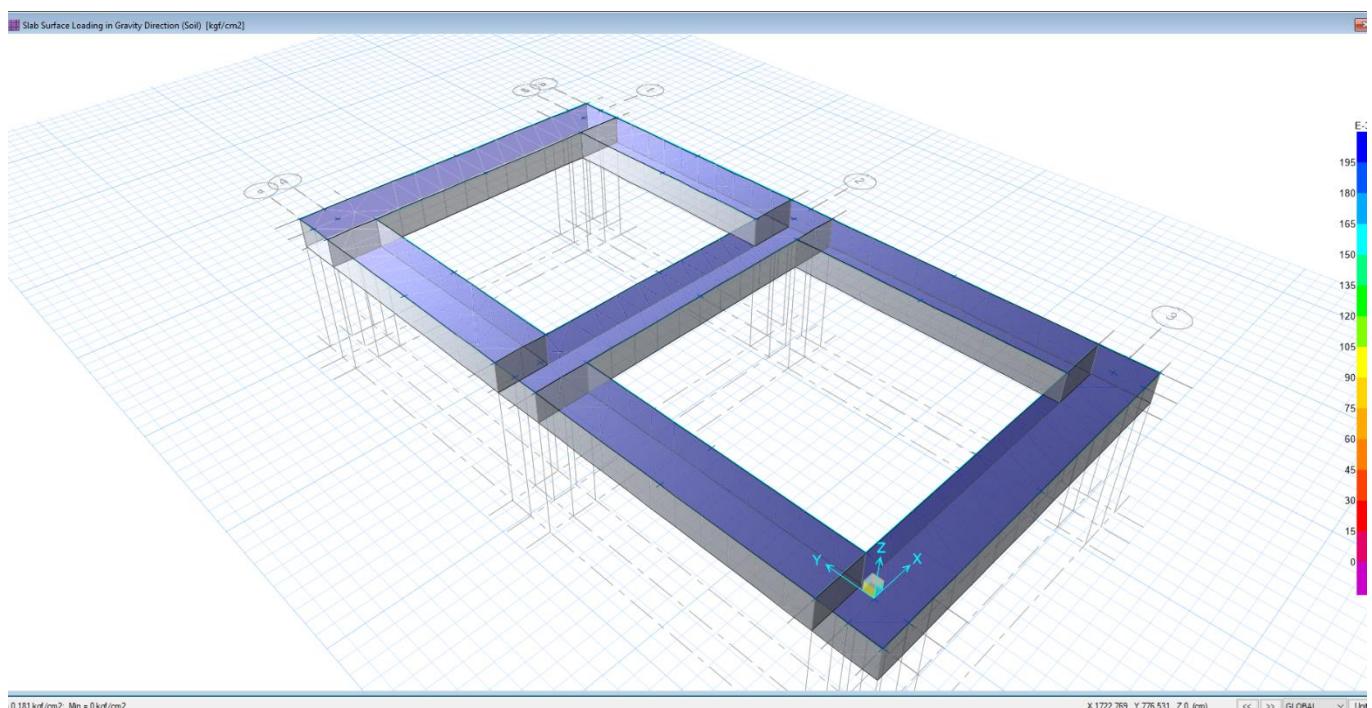


Figure 12: soil weight on foundation

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Calculation Note For CCTV Control Room- Binak GCS

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SETTLEMENT CONTROL

Settlement in different service load combinations should be checked by allowable value.

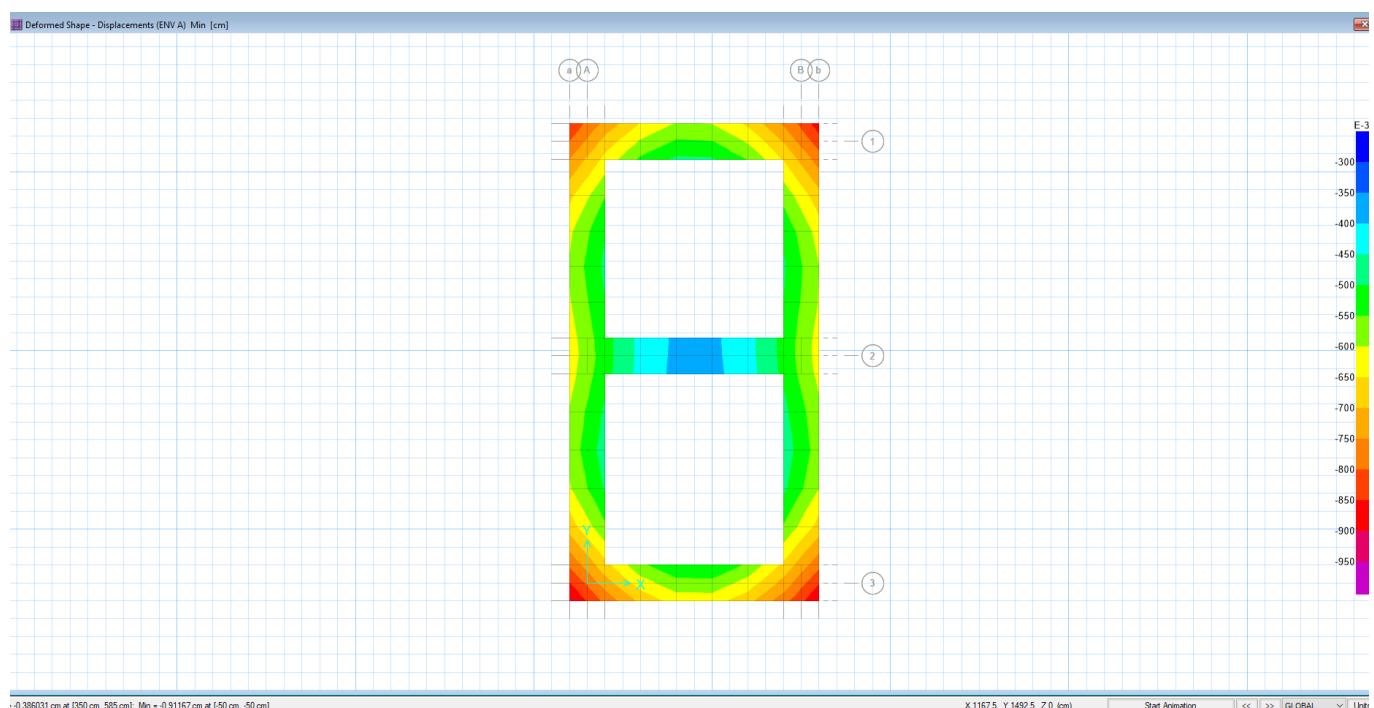


Figure 13: Foundation Settlement

Maximum settlement of foundation equals to 0.91 cm, which is less than allowable 1.6 cm.

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Calculation Note For CCTV Control Room- Binak GCS

شماره صفحه: ۲۸ از ۳۰

○ **SOIL PRESSURE CONTROL**

Soil pressures in different service load combinations should be checked by allowable value.

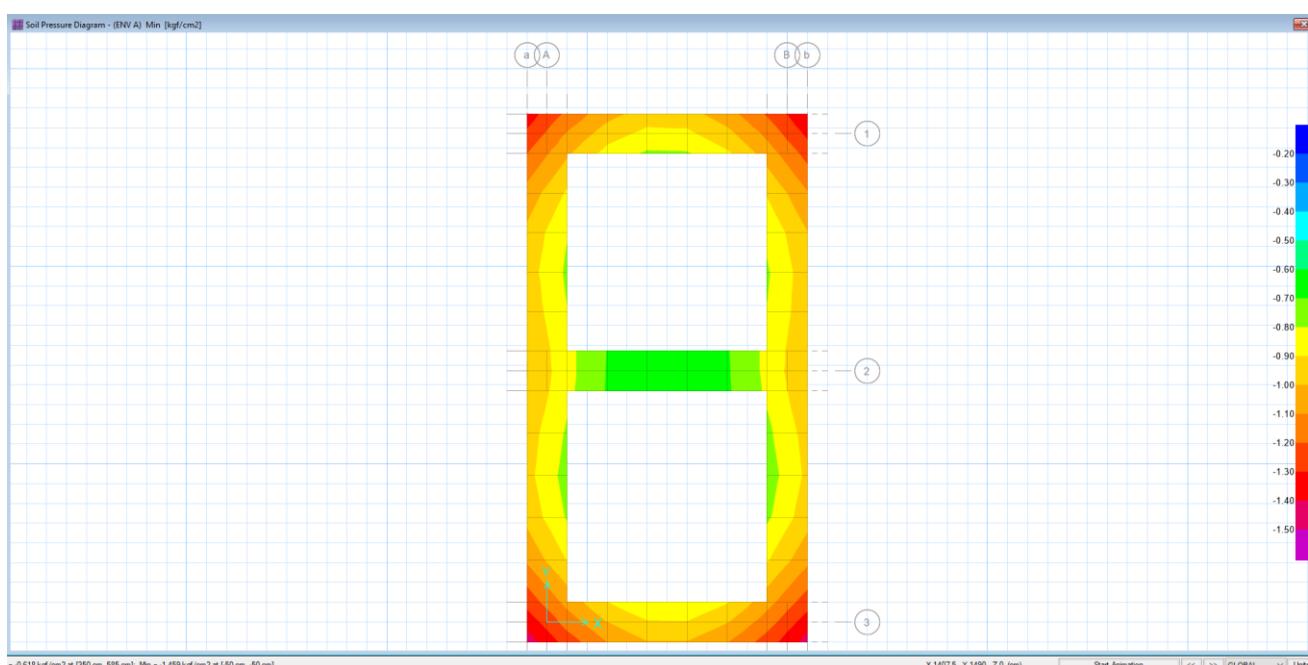
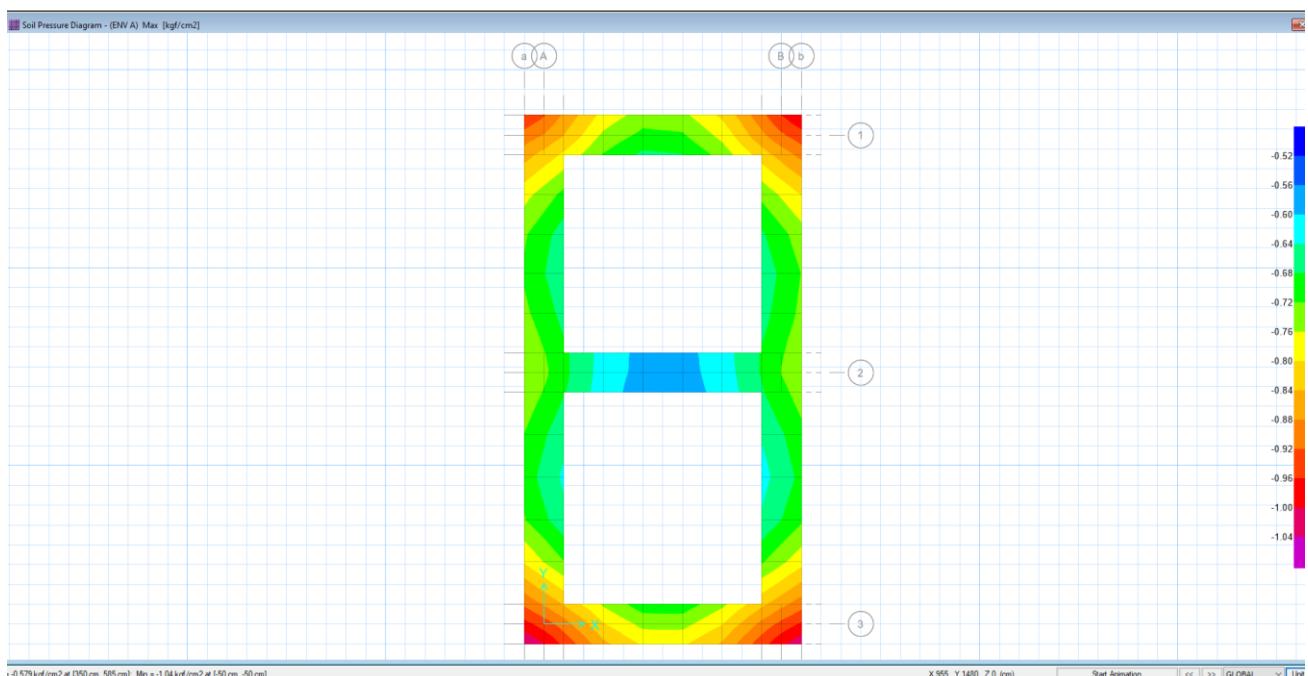


Figure 14: Soil Pressure under Foundation

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Calculation Note For CCTV Control Room- Binak GCS

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Maximum soil pressure under foundation equals to 1.43 kg/cm^2 , which is less than 1.75 kg/cm^2 .

○ PUNCHING SHEAR CONTROL

The punching shear control ratio of foundation is shown below. As seen the punching shear ratio in all columns base which is calculated by software is less than allowable range (1.0), so the footing thickness is acceptable.

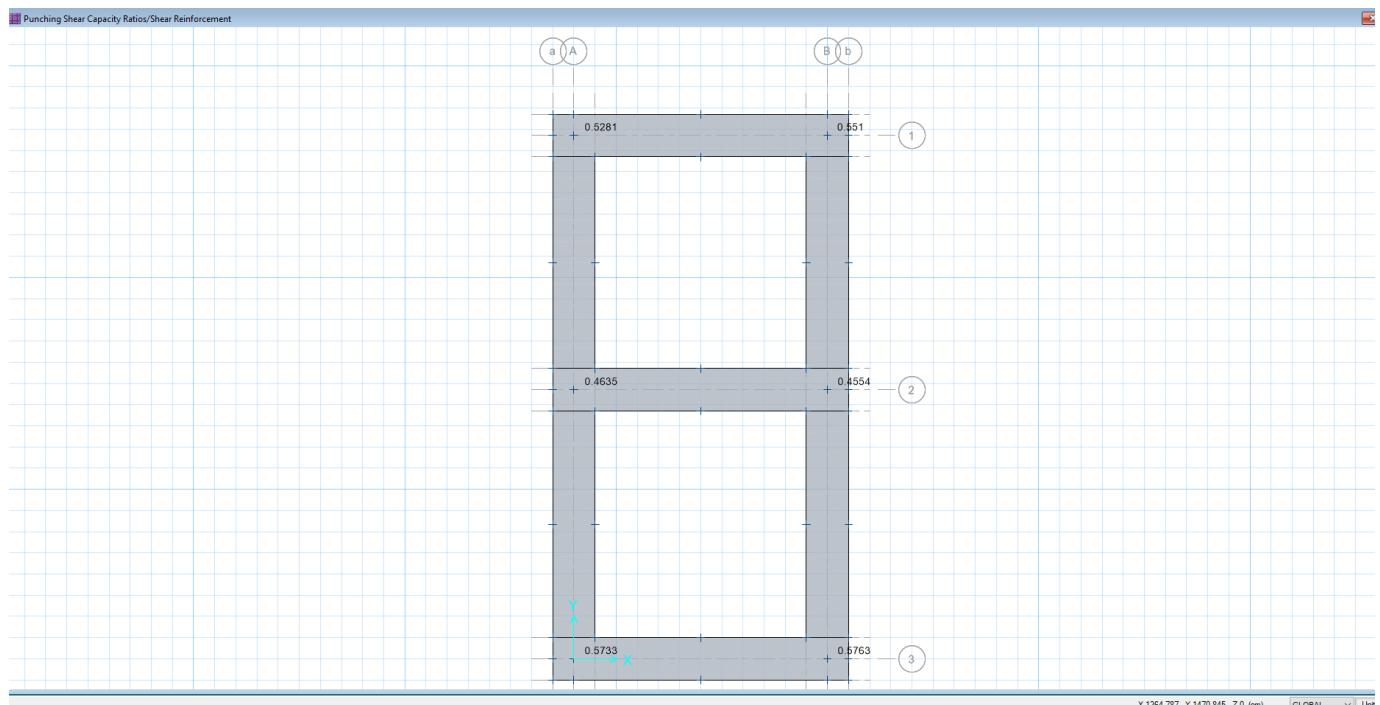


Figure 15: Punching Shear Capacity Ratios

○ FOUNDATION DESIGN

Foundation reinforcement is calculated by software and add bars in X, Y direction are shown at below figure. Uniform $\Phi 16@150$ pattern is assigned for top and bottom of foundation in both directions and needed additional bars at some part that is presented in below pictures.

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Calculation Note For CCTV Control Room- Binak GCS

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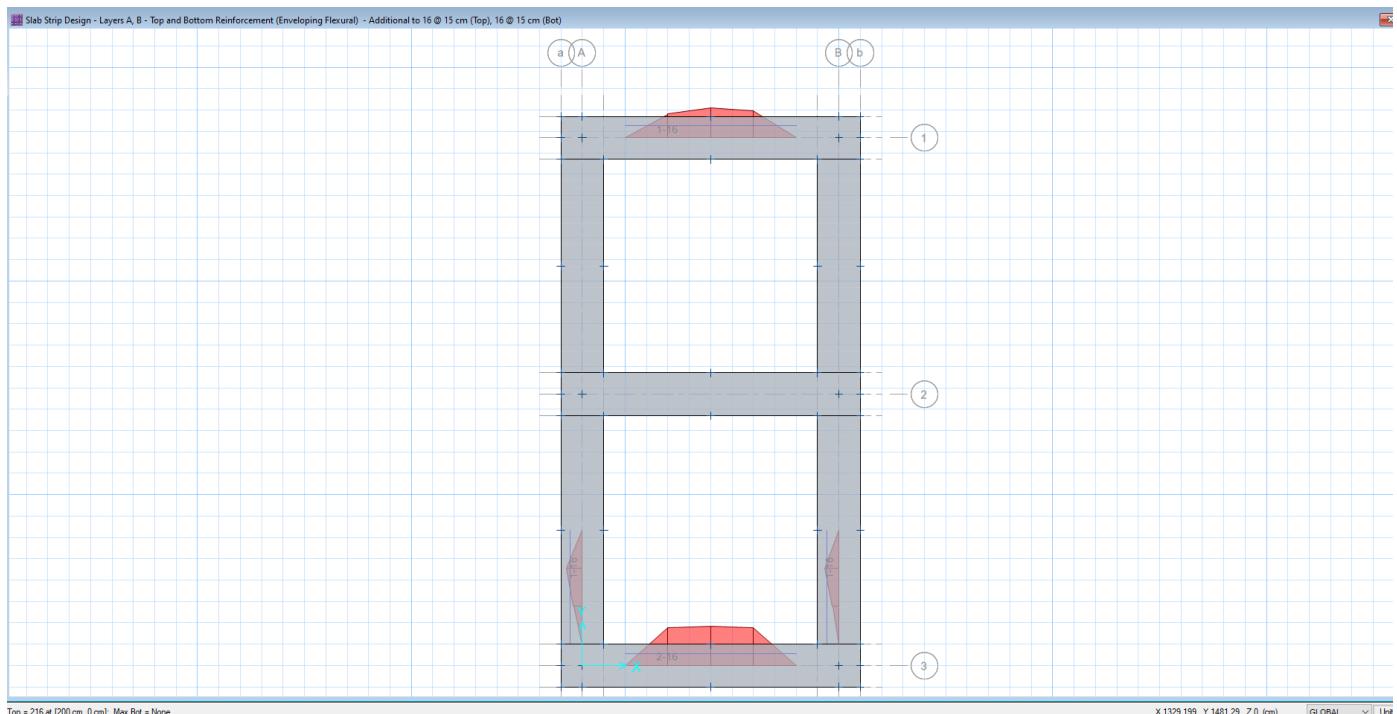


Figure 16: Reinforcement in X, Y Direction

In Both Directions:

Top Bar USE $\Phi 16 @ 150$ mm

Bottom Bar USE $\Phi 16 @ 150$ mm

Minimum rebar for strip foundation:

$$A_{s\min} = 0.0018 \times b \times h = 0.0018 \times 100 \times 60 = 10.8 \text{ cm}^2/\text{m}$$

$$A_{s\text{ used}} = \Phi 16 @ 150 = 12.06 \text{ cm}^2$$