



## نگهداشت و افزایش تولید میدان نفتی بینک سطح الارض و ابنيه تحت الارض

#### احداث ردیف تراکم گاز در ایستگاه جمع آوری پینک



شماره پیمان:

## **Calculation Note For Special Pipe Supports**

شماره صفحه : 1 از 18

K GCS PEDCO 120 ST CN 0016 D00

## طرح نگهداری و افزایش تولید 27 مخزن

## CALCULATION NOTE FOR SPECIAL PIPE SUPPORTS

## نگهداری و افزایش تولید میدان نفتی پینک

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**Status:**

- IDC:** Inter-Discipline Check  
**IFC:** Issued For Comment  
**IFA:** Issued For Approval  
**AFD:** Approved For Design  
**AFC:** Approved For Construction  
**AFP:** Approved For Purchase  
**AFQ:** Approved For Quotation  
**IFI:** Issued For Information  
**AB-R:** As-Built for CLIENT Review  
**AB-A:** As-Built –Approved

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## 1.0 INTRODUCTION

Binak oilfield in Bushehr province is a part of the southern oilfields of Iran, is located 20 km northwest of Genaveh city.

With the aim of increasing production of oil from Binak oilfield, an EPC/EPD Project has been defined by NIOC/NISOC and awarded to Petro Iran Development Company (PEDCO). Also PEDCO (as General Contractor) has assigned the EPC-packages of the Project to "Hirgan Energy - Design and Inspection" JV.

## 2.0 SCOPE

This report covers the structure & foundation calculation report of the "Chemical injection and Storage Shelter". The structure modelled by "SAP" software & the foundation modelled by "SAP" software too.

## 3.0 NORMATIVE REFERENCE

### 3.1 Local Codes and Standards

- INBC Part 6 "Iranian National Building Code"
- INBC Part 7 "Iranian National Building Code"
- INBC Part 9 "Iranian National Building Code"
- INBC Part 10 "Iranian National Building Code"
- Iranian Seismic Design Code for Petroleum Facilities(3<sup>rd</sup> edition)

### 3.2 International Codes and Standards

- ASCE 7-10 "Minimum Design Loads and Associated Criteria for Buildings and Other Structures-American Society of Civil Engineers".
- ACI 318. "Building Code Requirements for Reinforced Concrete", American Concrete Institute.
- AISC 358 "Prequalified Connections for Special and Intermediate Steel Moment Frames for Seismic Applications." American Institute of Steel Construction, Inc.
- AISC 360 - "Specification for Structural Steel Buildings". American Institute of Steel Construction, Inc.

### 3.3 The Project Documents

- BK-GNRAL-PEDCO-000-ST-SP-0001 SPECIFICATION FOR CONCRETE WORK
- BK-GCS-PEDCO-120-ST-DW-0058 STRUCTURAL DRAWING FOR CHEMICAL INJECTION &

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## STORAGE SHELTER

5.2 BK-GNRAL-PEDCO-000-ST-DC-0001 STRUCTURAL DESIGN CRITERIA

5.3 BK-GNRAL-PEDCO-000-CV-SP-0004 SPECIFICATION FOR EARTH WORK

5.4 BK-GCS-PEDCO-120-GT-RT-0001 GEOTECHNICAL INVESTIGATION REPORT FOR

## COMPRESSOR STATION

## 4.0 MATERIAL PROPERTIES

Material properties are delivered in the following table.

TABLE 1 -MATERIAL PROPERTIES

Foundation Concrete	F'c = 30 Mpa(28- day cylindrical sample)
Long. reinforcement bar	Fy = 400 Mpa(AIII)
Trans. reinforcement bar	Fy = 400 Mpa(AIII)
Bolt Type	HV 8.8
Electrode Type	E 70
Structural Steel shapes and plates:	St 37(Fy=2400kg/cm <sup>2</sup> , Fu=3700 kg/cm <sup>2</sup> )

## 5.0 STRUCTURE 'S SYSTEMS

The structure's system is Inverted pendulum.

TABLE 2 -STRUCTURAL SYSTEM

	SYSTEM	R	OMEGA	CD
X DIR	PENDULUM	2	2	2
Y DIR	PENDULUM	2	2	2

## 6.0 INPUT DATA

Structural elements stability and stress check have been performed considering the following load conditions:

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LoadPat	DesignType	SelfWtMult	Notes
DL	Dead	1	Weight of the Structural members and permanent attachments or accessories
Test	Dead	0	The empty weight of the pipes plus the weight of the test medium
LL	Live	0	Live Load
OPR1	Dead	0	The weight of piping, piping insulation, cable tray, process equipment and vessels plus their contents (fluid load) in Thermal load. Operating Load=DLempty+LLop
EQX	Quake	0	Earthquake load is determined by equivalent static earthquake analysis at axis X
EQY	Quake	0	Earthquake load is determined by equivalent static earthquake analysis at axis Y
OCX	Quake	0	Pure seismic load of piping at axis X
TL	Other	0	Thermal loads are those forces caused by a change in temperature. Such forces shall include those caused by vessel or piping expansion or contraction.
OCY	Quake	0	Pure seismic load of piping at axis Y
FRX	Other	0	Friction loads due to thermal expansion of pipes axis X
FRY	Other	0	Friction loads due to thermal expansion of pipes axis Y
EQZ	Quake	0	Vertical Earthquake load
TLst	Temperature	0	Temperature Load
NotionalX(DL)	Notional	0	Notional load derived from DL load at axis X
NotionalY(DL)	Notional	0	Notional load derived from DL load at axis Y
NotionalX(OPR)	Notional	0	Notional load derived from OPR load at axis X
NotionalY(OPR)	Notional	0	Notional load derived from OPR load at axis Y
NotionalX(Test)	Notional	0	Notional load derived from Test load at axis X
NotionalY(Test)	Notional	0	Notional load derived from Test load at axis Y
Soil	Dead	0	Soil load

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## 7.0 DESIGN LOAD

### 7.1 Dead load

The self-weight of structural elements (introduced Dead Load/DL in SAP) is automatically considered by SAP program.

### 7.2 OPERATION LOADS

Operation load of pipes according to piping documents assign in model.

### 7.3 HYDROTEST LOADS

Hydrotest load of pipes according to piping documents assign in model.

### 7.4 PURE SEISMIC WEIGHT OF PIPING (OCX & OCY)

Seismic loads of pipes according to piping documents assign in model. In some cases when the earthquake load is not provided by piping, the earthquake coefficient is calculated by Iranian seismic design code for Petroleum facilities (3rd edition). the same behavior as the inverted pendulum.

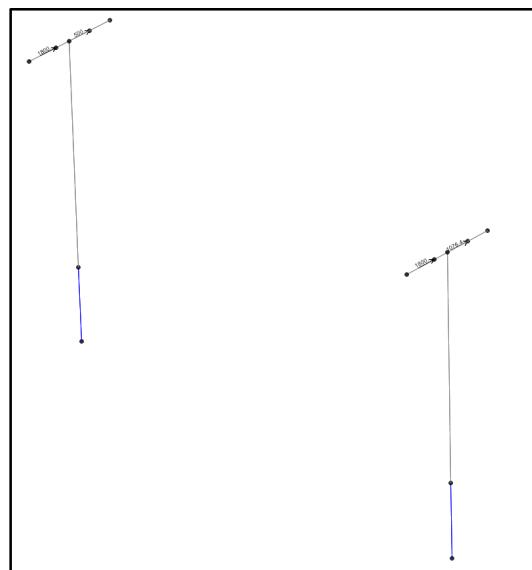


FIGURE 1. PURE SEISMIC WEIGHT OF PIPING (OCX)

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## 7.5 SEISMIC LOAD (EQX,EQY)

All structures are in area with high risk zone of seismic and until finalizing of "Geotechnical Final Report" soil type consider is type II. Equivalent static method is used for calculation of seismic loads. Parameters which are used in calculation of earthquake force and seismic coefficient is presented in below According to Iranian seismic design code for Petroleum facilities (3rd edition). Structures with the same behavior as the inverted pendulum.

$$V = \frac{S_a}{R/I_e} W$$

Where:

$C_s$  = the seismic response coefficient from Equation below:

$W$  = the effective seismic weight of the structure

$$C_s = \frac{S_a}{R/I_e}$$

Where:

$S_a$ = mapped spectral response acceleration parameter (g), determined from hazard analysis.

$$S_{DS} = 0.75$$

$$S_{D1} = 0.375$$

$R$ = the response modification factor for structure (Structures with the same behavior as the inverted pendulum)

$I$  = the importance factor for structure:1.25

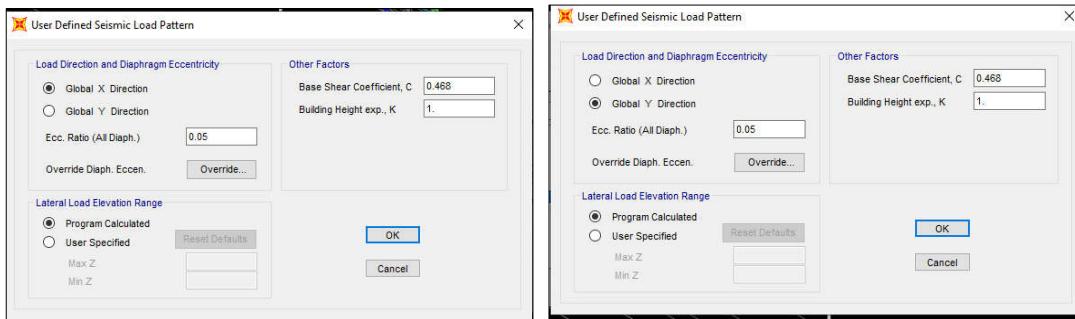
$R_{uy}=2$

$\Omega=2$

$C_d=2$

$$C_s = \frac{0.75 \times 1.25}{2} = 0.468$$

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**FIGURE 2. PERIOD IN X,Y DIRECTION**

▪ **Vertical seismic component:**

The vertical seismic load effect,  $E_v$ , shall be determined in accordance with the following Equation 6):

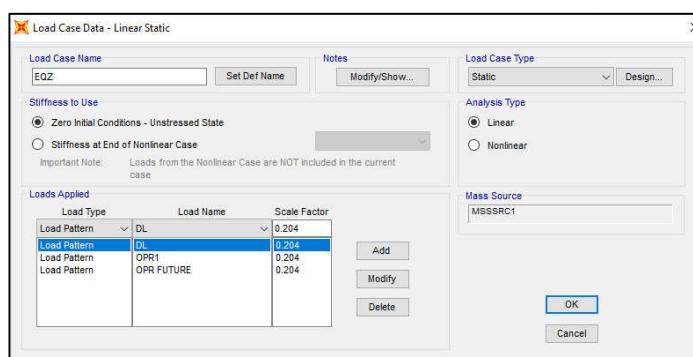
$$E_v = 0.2S_{DS}D$$

$S_{DS}$  = Design, 5% damped, spectral response acceleration parameter (g) at short periods (0.2 sec).

D = effect of dead load

Loads case name: EQZ=0.2×0.75×W=0.15×W

-Ev : Vertical seismic load applied at model:



**FIGURE 3. APPLIED EV LOAD**

Ev applied at model as a portion of dead load as above.

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## 7.6 FRICTION LOADS (FRX, FRY)

Based on design criteria on pipe racks and pipe supports with 3 or less lines the friction force shall be taken as 30% of the total pipe weight under operating condition.

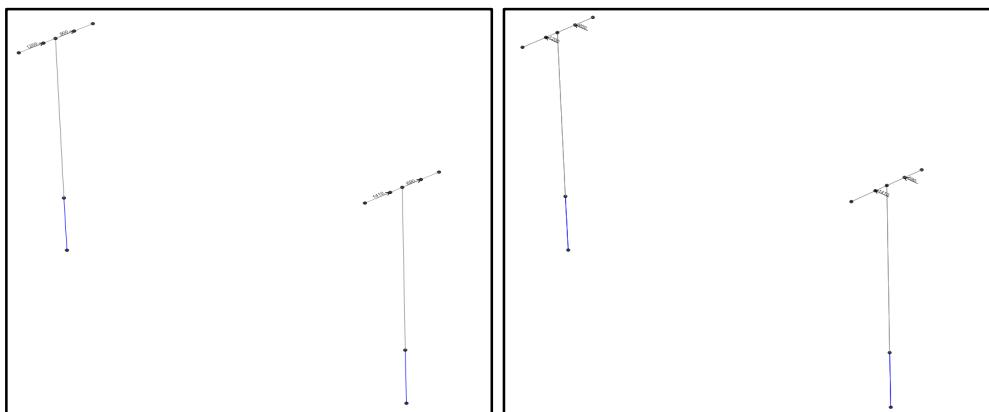


FIGURE 4. FRICTION LOADS (FRX, FRY)

## 7.7 THERMAL LOAD OF STRUCTURE (TLST)

According to "Specification for Civil and Structural Design Criteria". Maximum temperature of 28 °C shall be considered for computing the thermal load in all components.

### 1.1 SAP LOADING TABLE

- ✓ Load pattern:

TABLE: Load Pattern Definitions				
LoadPat	DesignType	SelfWtMult	AutoLoad	NotBasePat
Text	Text	Unitless	Text	Text
DL	Dead	1		
OPR1	Dead	0		
FRX	Other	0		
FRY	Other	0		
EQX	Quake	0	USER COEFF	
EQY	Quake	0	USER COEFF	
EQZ	Quake	0	None	
TL	Other	0		
L	Live	0		
Test	Dead	0		
Soil	Dead	0		
OCX	Quake	0	None	

 NISOC	<b>نگهداشت و افزایش تولید میدان نفتی بینک</b> <b>سطح الارض و ابنيه تحت الارض</b> <b>احداث ردیف تراکم گاز در ایستگاه جمع آوری بینک</b>	 هیرگان انرژی
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TABLE: Load Pattern Definitions						
LoadPat	DesignType	SelfWtMult	AutoLoad	NotBasePat		
Text	Text	Unitless	Text	Text		
OCY	Quake	0	None			
NotionalX(DL)	Notional	0		DL		
NotionalY(DL)	Notional	0		DL		
NotionalX(OPR)	Notional	0		OPR1		
NotionalY(OPR)	Notional	0		OPR1		
NotionalX(Test)	Notional	0		Test		
NotionalY(Test)	Notional	0		Test		

## 8.0 Load combinations

According to code INBC No.60(4 th edition) structures, components, and foundations shall be designed, so that their design strength equals or exceeds that effect of factored loads in the following combination:

- 1.4(D)
- (1.2D) +1.6(L)+0.5(Lr/S/R)
- 1.2D+1.6(Lr/S/R) + (L/0.5W)
- 1.2D+1.0(W) + L+.5(Lr/S)
- 1.2D+1.0E+L+0.2S
- 0.9D+1.0W
- 0.9D+1.0E

Load listed herein shall be considered to act in the following combinations; whichever produces the most unfavorable effect considering soil reactions.

- D
- D+L
- D+(Lr/S/R)
- D+0.75(L)+0.75(Lr/R/S)
- D+(0.6W or 0.7E)
- D+0.75L+0.75(0.6W)+0.75(Lr/S/R)
- D+0.75L+0.75(0.7E)+0.75S
- 0.6D+0.6W
- 0.6D+0.7E

## 9.0 STRUCTURE ANALYSIS AND DESIGN

### 9.1 ANALYSIS

Structural analysis is done by SAP2000 software. In model loads are applied, some graphical outputs from model are shown as follows.

 <b>NISOC</b>	<b>نگهداشت و افزایش تولید میدان نفتی بینک</b> <b>سطح الارض و ابنيه تحت الارض</b> <b>احداث ردیف تراکم گاز در ایستگاه جمع آوری بینک</b>	 <b>hirgan ENERGY</b>
شماره پیمان: <b>053 - 073 - 9184</b>	<b>Calculation Note For Special Pipe Supports</b>	شماره صفحه : 12 از 18

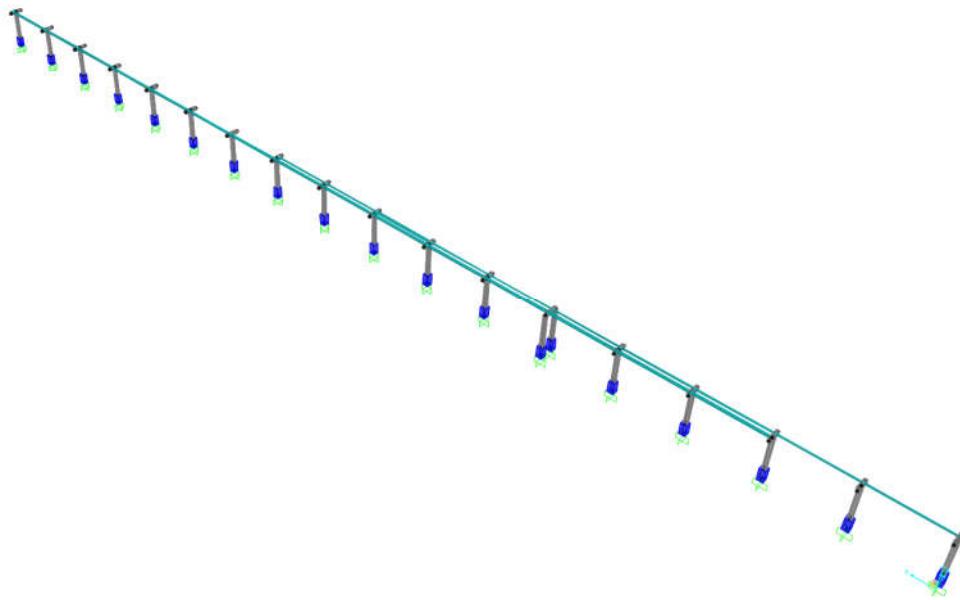


FIGURE 5. 3D VIEW OF SAP MODEL

## 9.2 Base Plate



Current Date: 2/27/2024 1:21 PM

Units system: Metric

File name: C:\ProgramData\Bentley\Engineering\RAM Connection\13.7.0\Data\Base Plate Examples.rcnx

### Steel connections

#### Results

Connection name : Pinned BP  
 Connection ID : 1

Family: Column - Base (CB)  
 Type: Base plate  
 Description: DG1 LRFD - Example 4.1  
 Design code: AISC 360-05 LRFD, ACI 318-08

DEMANDS		P <sub>u</sub> [T]	M <sub>u22</sub> [T*m]	M <sub>u33</sub> [T*m]	V <sub>u2</sub> [T]	V <sub>u3</sub> [T]	Load type
DC1		-25.00	0.00	0.00	5.00	3.00	Design

 <b>NISOC</b>	<b>نگهداشت و افزایش تولید میدان نفتی بینک</b> <b>سطح الارض و ابنيه تحت الارض</b> <b>احداث ردیف تراکم گاز در ایستگاه جمع آوری بینک</b>	 <b>HIRGAN ENERGY</b>
شماره پیمان: <b>053 - 073 - 9184</b>	<b>Calculation Note For Special Pipe Supports</b>	شماره صفحه : <b>13</b> از <b>18</b>

**Design for major axis**  
**Base plate (AISC 360-05 LRFD)**

**GEOMETRIC CONSIDERATIONS**

Dimensions	Unit	Value	Min. value	Max. value	Sta.	References
<u>Base plate</u>						
Distance from anchor to edge	[cm]	5.80	0.64	--	✓	
Weld size	[1/16in]	7	3	--	✓	table J2.4

**DESIGN CHECK**

Verification	Unit	Capacity	Demand	Ctrl EQ	Ratio	References
<u>Pedestal</u>						
Axial bearing	[Ton/cm <sup>2</sup> ]	0.17	0.01	DC1	0.04	DG1 3.1.1;
<u>Base plate</u>						
Flexural yielding (bearing interface)	[Ton*m/m]	3.56	1.06	DC1	0.30	DG1 Eq. 3.3.13
Flexural yielding (tension interface)	[Ton*m/m]	3.56	0.00	DC1	0.00	DG1 Eq. 3.3.13
<u>Column</u>						
Weld capacity	[Ton/m]	261.03	0.00	DC1	0.00	p. 8-9, Sec. J2.5, Sec. J2.4
Elastic method weld shear capacity	[Ton/m]	174.02	8.33	DC1	0.05	p. 8-9, Sec. J2.5, Sec. J2.4
Elastic method weld axial capacity	[Ton/m]	261.03	0.00	DC1	0.00	p. 8-9, Sec. J2.5, Sec. J2.4
<b>Ratio</b>	<b>0.30</b>					

**Major axis**  
**Anchors**

**GEOMETRIC CONSIDERATIONS**

Dimensions	Unit	Value	Min. value	Max. value	Sta.	References
<u>Anchors</u>						
Anchor spacing	[cm]	23.00	9.60	--	✓	Sec. D.8.1
Concrete cover	[cm]	13.30	5.08	--	✓	Sec. 7.7.1
Effective length	[cm]	51.56	--	138.44	✓	

**DESIGN CHECK**

Verification	Unit	Capacity	Demand	Ctrl EQ	Ratio	References
<u>Anchor tension</u>						
Breakout of anchor in tension	[Ton]	10.80	0.00	DC1	0.00	Eq. D-3
Breakout of anchor in tension	[Ton]	12.34	0.00	DC1	0.00	Eq. D-4, Sec. D.4.1.1
<u>Pullout of anchor in tension</u>						
Side-face blowout of anchor in tension	[Ton]	15.77	0.00	DC1	0.00	Sec. D.4.1.1
Side-face blowout of anchor in tension	[Ton]	9.66	0.00	DC1	0.00	Sec. D.5.4.1, Sec. D.4.1.1
<u>Anchor shear</u>						
Anchor shear	[Ton]	4.49	0.83	DC1	0.19	Eq. D-20, Sec. D.6.1.3
<u>Breakout of anchor in shear</u>						
Breakout of anchor in shear	[Ton]	3.06	0.83	DC1	0.27	Sec. D.4.1.1
Breakout of group of anchors in shear	[Ton]	10.76	5.00	DC1	0.46	Sec. D.4.1.1
Pryout of anchor in shear	[Ton]	24.68	0.83	DC1	0.03	Eq. D-4, Sec. D.4.1.1
<b>Ratio</b>	<b>0.46</b>					
<b>Global critical strength ratio</b>	<b>0.46</b>					

**Major axis**



نگهداری و افزایش تولید میدان نفتی بینک  
سطح الارض و ابنيه تحت الارض

## احداث ردیف تراکم گاز در ایستگاه جمع آوری بینک

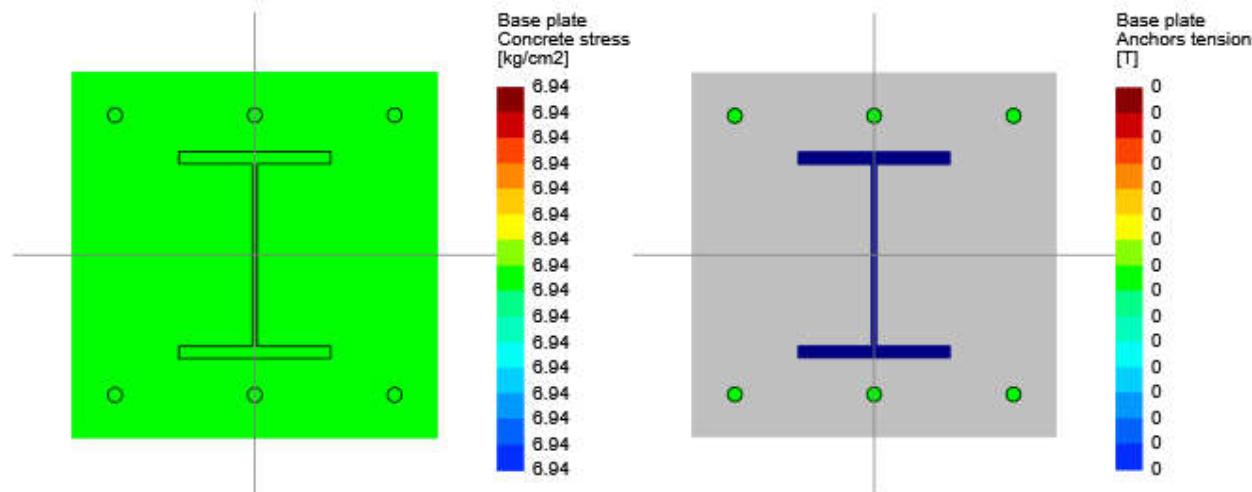


شماره پیمان:  
053 - 073 - 9184

### Calculation Note For Special Pipe Supports

شماره صفحه: 14 از 18

#### Maximum compression and tension (DC1)



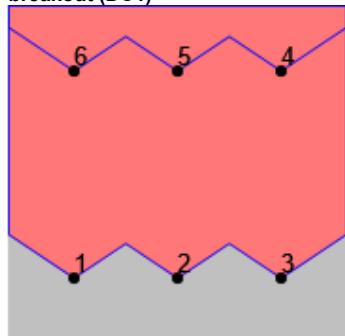
Maximum bearing pressure	6.94	[kg/cm <sup>2</sup> ]
Minimum bearing pressure	6.94	[kg/cm <sup>2</sup> ]
Maximum anchor tension	0.00	[T]
Minimum anchor tension	0.00	[T]
Neutral axis angle	0.00	[deg]
Bearing length	1E32	[cm]

#### Anchors tensions

Anchor	Transverse [cm]	Longitudinal [cm]	Shear [T]	Tension [T]
1	-23.00	-23.00	0.83	0.00
2	0.00	-23.00	0.83	0.00
3	23.00	-23.00	0.83	0.00
4	23.00	23.00	0.83	0.00
5	0.00	23.00	0.83	0.00
6	-23.00	23.00	0.83	0.00

#### Major axis

##### Results for shear breakout (DC1)



Group	Area [cm <sup>2</sup> ]	Shear [T]	Anchors
1	6806.25	5.00	1, 2, 3, 4, 5, 6
2	1631.25	2.50	4, 5, 6

 <b>NISOC</b>	<b>نگهداشت و افزایش تولید میدان نفتی بینک</b> <b>سطح الارض و ابنيه تحت الارض</b> <b>احداث ردیف تراکم گاز در ایستگاه جمع آوری بینک</b>	 <b>HIRGAN ENERGY</b>																
شماره پیمان: 053 - 073 - 9184	<b>Calculation Note For Special Pipe Supports</b> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>بروژه</th><th>بسته کاری</th><th>صادرکننده</th><th>تمیلات</th><th>رشته</th><th>نوع مدرک</th><th>سربال</th><th>نسخه</th></tr> </thead> <tbody> <tr> <td>BK</td><td>GCS</td><td>PEDCO</td><td>120</td><td>ST</td><td>CN</td><td>0016</td><td>D00</td></tr> </tbody> </table>	بروژه	بسته کاری	صادرکننده	تمیلات	رشته	نوع مدرک	سربال	نسخه	BK	GCS	PEDCO	120	ST	CN	0016	D00	شماره صفحه : 15 از 18
بروژه	بسته کاری	صادرکننده	تمیلات	رشته	نوع مدرک	سربال	نسخه											
BK	GCS	PEDCO	120	ST	CN	0016	D00											

## 10.0 FOUNDATION DESIGN

SAFE has been used in order to modeling, analyses and design of this foundation.

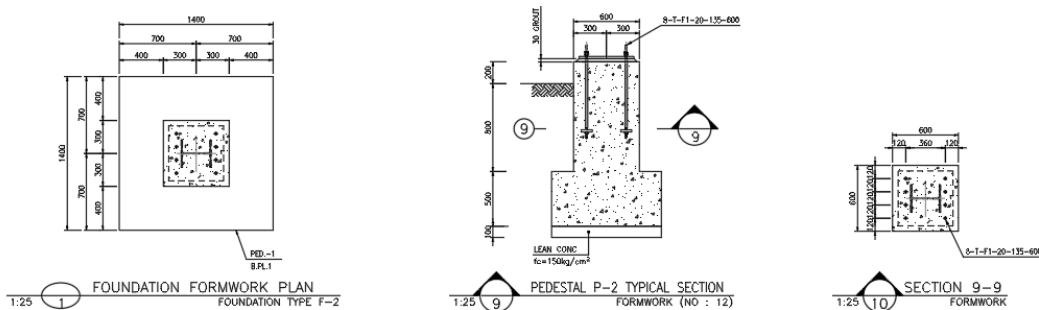


FIGURE 6. FOUNDATION PLAN

### 10.1 Soil pressure and settlement

Until finalize of geotechnical report for this area we consider  $\Rightarrow q_a = 2 \text{ kg/cm}^2$

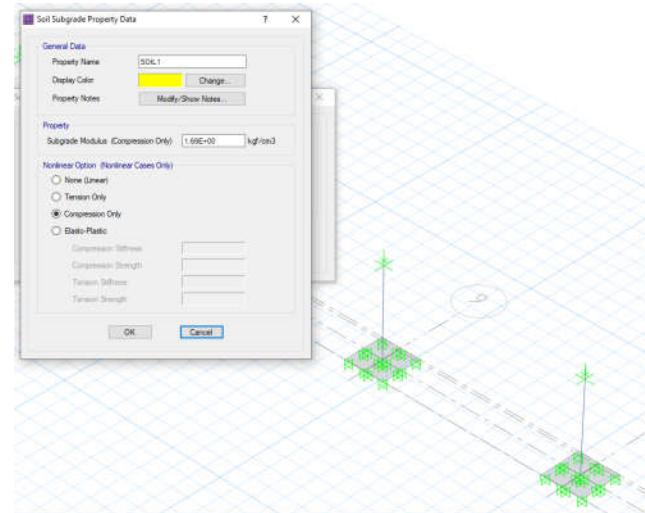
Based on geotechnical report for subgrade modulus is  $\Rightarrow K_s = 1.69 \text{ kg/cm}^3$

$B(m)$	مدول عکس العمل پستر بی سطحی ( $\text{kg/cm}^3$ )			
	$L/B=1$	$L/B=2$	$L/B=5$	$L/B=10$
1.0	1.69	1.33	1.14	1.09
2.0	1.00	0.85	0.78	0.77
3.0	0.78	0.69	0.65	0.64
4.0	0.68	0.60	0.57	0.57
5.0	0.61	0.55	0.53	0.53

FIGURE 7. SUBGRADE MODULUS

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پروژه	بسته کاری	صادرکننده	تمهیلات	رشته	نوع مدرک	نوع سریال	نسخه
BK	GCS	PEDCO	120	ST	CN	0016	D00



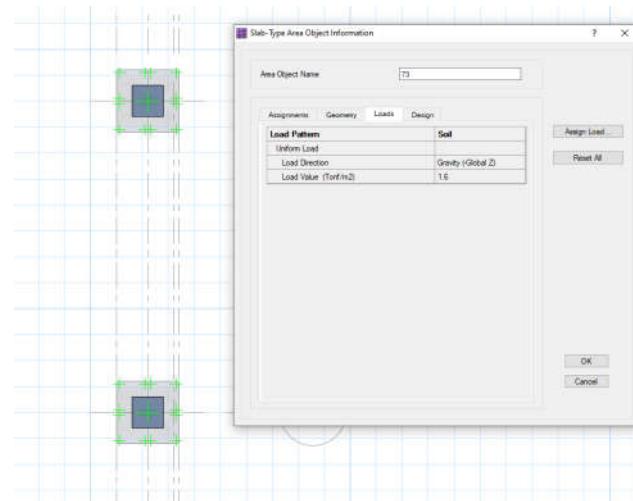
**FIGURE 8. ASSIGN SPRING TO FOUNDATION**

## 10.2 DESIGN

Concrete Foundation are designed according to ACI 318-14. Required loads are derived from SAP data, and design process will be done according to ACI code based on ultimate strength procedure.

$$f'_c = 30 \text{ MPa} \quad f_y = 400 \text{ MPa}$$

Soil load applied on model as below:



**FIGURE 9. APPLIED SOIL LOAD ON FOUNDATION**

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$$\text{Soil dead load is } \gamma h = 1.85 * 0.6 = 1.11 \frac{\text{ton}}{\text{m}^2} = 1110 \frac{\text{kg}}{\text{m}^2}$$

## 10.3 FOUNDATION DESIGN CONTROL

### 12.3.1 CHECK OF STRESS FOR FOUNDATION

TABLE: Nodal Displacements						
Point	OutputCase	CaseType	Ux	Uy	Uz	
71	COMB11	Combination	0	0	-0.12303	
239	COMB11	Combination	0	0	-0.12303	
71	ENV	Combination	0	0	-0.12303	
239	ENV	Combination	0	0	-0.12303	
119	COMB11	Combination	0	0	-0.1227	
119	ENV	Combination	0	0	-0.1227	
118	COMB11	Combination	0	0	-0.12267	
123	COMB11	Combination	0	0	-0.12267	
118	ENV	Combination	0	0	-0.12267	
123	ENV	Combination	0	0	-0.12267	
117	COMB11	Combination	0	0	-0.12264	
117	ENV	Combination	0	0	-0.12264	
121	COMB11	Combination	0	0	-0.12264	
124	COMB11	Combination	0	0	-0.12264	
121	ENV	Combination	0	0	-0.12264	
124	ENV	Combination	0	0	-0.12264	
116	COMB11	Combination	0	0	-0.12258	
122	COMB11	Combination	0	0	-0.12258	
116	ENV	Combination	0	0	-0.12258	
122	ENV	Combination	0	0	-0.12258	
22	COMB11	Combination	0	0	-0.11921	
232	COMB11	Combination	0	0	-0.11921	
22	ENV	Combination	0	0	-0.11921	
232	ENV	Combination	0	0	-0.11921	
51	COMB11	Combination	0	0	-0.1189	
51	ENV	Combination	0	0	-0.1189	
52	COMB11	Combination	0	0	-0.11886	
56	COMB11	Combination	0	0	-0.11886	
52	ENV	Combination	0	0	-0.11886	
56	ENV	Combination	0	0	-0.11886	
49	COMB11	Combination	0	0	-0.11884	
55	COMB11	Combination	0	0	-0.11884	

 <b>NISOC</b>	<b>نگهداشت و افزایش تولید میدان نفتی بینک</b> <b>سطح الارض و ابنيه تحت الارض</b> <b>احداث ردیف تراکم گاز در ایستگاه جمع آوری بینک</b>	 <b>هیرگان انرژی</b>
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TABLE: Nodal Displacements						
Point	OutputCase	CaseType	Ux	Uy	Uz	
49	ENV	Combination	0	0	-0.11884	
55	ENV	Combination	0	0	-0.11884	
			MAX	-0.12303		

According to above output, Max soil pressure under the foundation is:

$$q_n = 0.123 \times 1.69 = 0.200 \text{ kg/cm}^2 < 2 \text{ kg/cm}^2 \text{ ok}$$

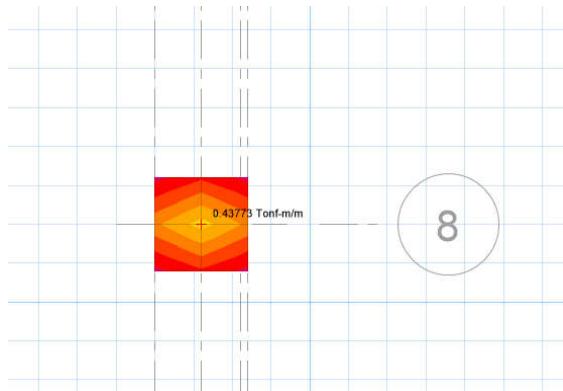
### 12.3.3 REINFORCING CONTROL

Minimum rebar for foundation:

$$A_{s\ min} = 0.0018bh$$

$$A_{smin} = \frac{1}{2} 0.0018 bh = \frac{1}{2} 0.0018 \times 100 \times 50 = 4.5 \text{ cm}^2/\text{m}$$

$$A_{s\ used} = \emptyset 16 @ 200 = 10.05 \text{ cm}^2$$



**FIGURE 10. M22 RESULT**